

**Characterization of Unsaturated Stony Vadose Zones Using
Standard Physical Methods**

by

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ABSTRACT

The relationship of unsaturated hydraulic conductivity to the volume percent of stones in a soil profile was the focus of this research. An additional objective was the evaluation of two physical measurement methods utilized in determining unsaturated hydraulic properties in stony soils. These methods were the Time Domain Reflectometry (TDR) technique for water content measurement and the instantaneous profile method for determination of unsaturated hydraulic conductivity. Homogeneous and heterogeneous soil profiles were studied in a series of column experiments to determine their unsaturated hydraulic properties. Homogeneous profiles consisted of a fine uniform sand collected from a near by field area, while the heterogeneous profiles consisted of a single layer of hard composite spheres packed uniformly in a single sand layer of variable thickness. Volumetric water content and soil water tension measurements were collected and analyzed using the instantaneous profile method. The performance and accuracy of the TDR probes was also investigated to determine the suitability of this instrument for stony soils. The effectiveness of the TDR in stony soils is directly related to the representative elementary volume (REV) sampled by the instrument. The volume of soil sampled by the TDR probes used in this study was investigated empirically. Results of this research indicate that the unsaturated hydraulic conductivity of layered stony soils can be estimated accurately utilizing a simple correction for the volume of stones present in the soil profile. The instantaneous profile method, as applied to layered stony soils, must be corrected for the volume of stones present in the soil profile to avoid an overestimation of volumetric water content for the stony layers. Additionally, this method is limited in its ability to estimate hydraulic gradients in the region of impeding stone layers. Finally, when using the TDR technique

in heterogeneous soils containing stones the effective sample volume of the probes must be determined so that water content measurements may be quantified as being real matrix water contents or volumetric water contents to avoid errors in data analysis.

INTRODUCTION

The hydraulic properties of stony vadose zones are of great interest because of their wide spread occurrence in different geographic regions and manner in which they are utilized. The economic potential of stony soils is quite limited in regard to agricultural use or other land intensive developments. This means that these areas are often relegated to industrial use, landfills, or land treatment of waste water. Such practices carry a greater potential for soil and groundwater contamination than those of more economically desirable lands. As such, a better understanding of their hydraulic properties is essential to accurately predict the movement of water and dissolved contaminants in these systems.

A review of the current literature indicates a lack of information regarding the hydraulic properties of stony vadose zones. Only a limited number of studies describe, in a quantitative fashion, the effect that a given volume of stones has on unsaturated hydraulic conductivity. Mehuys et al.(1975) studied several desert soils and found that the apparent conductivities were higher for soils containing stones greater than one centimeter in diameter at volumetric water contents ranging from 5% to 20%. Bower and Rice (1985) developed an expression to estimate the bulk saturated hydraulic conductivity for stony soils using the measured value for the saturated matrix and the volume of stones present. This in turn may be used to estimate the unsaturated $K(\theta)$ or $K(\Psi)$ relationships for stony soils using a simple curve fitting procedure. The work of Brakensiek et al.(1994) provides equations for modifying soil/water physical properties in the presence of coarse rock fragments.

The reliability of the research methods used to obtain hydraulic characteristics was also evaluated

to determine their effectiveness in stony soils. We focused on the use of the Time Domain Reflectometry (TDR) technique as applied to the instantaneous profile method. Reports in the literature indicate that the area of influence affecting TDR measurements is confined to a region with maximum dimensions of 60 mm x 80 mm, with an asymmetrically distributed cross sectional area of 3600 mm² (Baker & Lascano 1989). This is for a two wire probe with wave guides 300 mm long having a separation distance of 50 mm. A similar area of influence was found by Topp and Davis (1985). It is further stated that the "effective" cross-sectional area is approximately 1000 mm² with maximum dimensions of 20 mm x 65 mm (Baker & Lascano 1989). The work of Knight et al. (1994), and Zegelin et al. (1992) suggest that this area of influence should be considerably less for a three wire probe. The area of influence for the three wire probes used in this study was empirically determined to assess the accuracy of measurements made in the stony layers.

The objective of this research is to investigate the effect that a given volume of stones, as a single layer, has on the hydraulic properties of a soil and to evaluate the effectiveness of the standard physical methods used in the collection of these data.

METHODS AND MATERIALS

A series of instantaneous profile experiments were conducted using both homogeneous and heterogeneous soil profiles in a number of large scale column experiments. The data were examined for direct relationships between the volume percent of stones in a profile and corresponding changes in the unsaturated hydraulic conductivity. Data from the heterogeneous profiles was further analyzed by comparing it to results fitted to the homogeneous profile data using the program RETC, van Genuchten et al. (1992).

Field Soil

Seven column experiments were conducted: three using columns packed with homogeneous sand and four using columns packed with layers of sand and rubber spheres. The sand was collected from the Sevilleta National Wild Life Refuge along the bank of the Rio Salado, an ephemeral stream, fifteen miles north of Socorro, New Mexico. This field sand consists of ancient flood plain deposits of the Rio Salado. The size distribution of this sand, based on sieve analysis data, is presented in Table 1. A series of hard composite rubber spheres, 6.4 cm in diameter, were used to represent stones in the heterogeneous profiles. These artificial stones were chosen because unlike natural materials their size and shape are uniform. This eliminates a significant number of variables from the experiment and allows a more direct assessment of the unsaturated hydraulic conductivity. The homogeneous sand columns established baseline hydraulic parameters for the field soil that was used, and served as a reference for the examination of layered profile experiments. Column size,

Sevilleta Field sand					
Sieve	Diameter	Weight	Cumulative	Percent (%)	Percent (%)
Number	(mm)	Retained	Weight	Passed	Retained
30	0.6	1.05	1.05	0.99927	0.00073
40	0.417	8.5	9.55	0.93316	0.06684
60	0.25	56.03	65.58	0.54095	0.45905
100	0.15	53.83	119.41	0.16415	0.83585
140	0.12	16.15	135.56	0.051099	0.948901
200	0.075	3.29	138.85	0.02807	0.97193
bottom		4.01	142.86	0	1

Table 1. Sieve analysis of the Sevilleta field soil.

shape, drainage conditions, and water application rates were all kept constant throughout all of the experiments. The following describes, in detail, the experimental setup utilized and presents the procedures used in conducting the experiments.

Soil Columns

The soil columns were constructed using a series of polyvinyl-chloride(PVC) plastic rings. Each ring was thirty centimeters in diameter and ten centimeters high. A total of fifteen rings were used in the construction of each column. Rings were placed on top of one another and the joints sealed with duct tape to prevent the leakage of water. This created a single rigid piece of pipe that served as the column, (Figure 1). A length of 1.5 meters was found to be sufficient for obtaining near unit gradient conditions in the homogeneous soil profiles. Each ring was given a numbered designation starting at the top of the column, which was taken as the ground surface datum, to facilitate data collection and record keeping. Six of the rings in the upper seventy centimeters were instrumented to collect soil water tension and volumetric water content measurements, (Figure 1). These measurement stations, rings two through seven, were positioned at regular intervals throughout the top portion of the column, spaced in an equidistant fashion ten centimeters apart. The single non-instrumented ring at the very top of the column and those at the bottom were positioned solely for maintaining the uniformity of wetting during infiltration, and to eliminate unwanted drainage effects near the last measurement station.

Column Packing

After the columns were assembled they were filled completely with either a homogeneous or

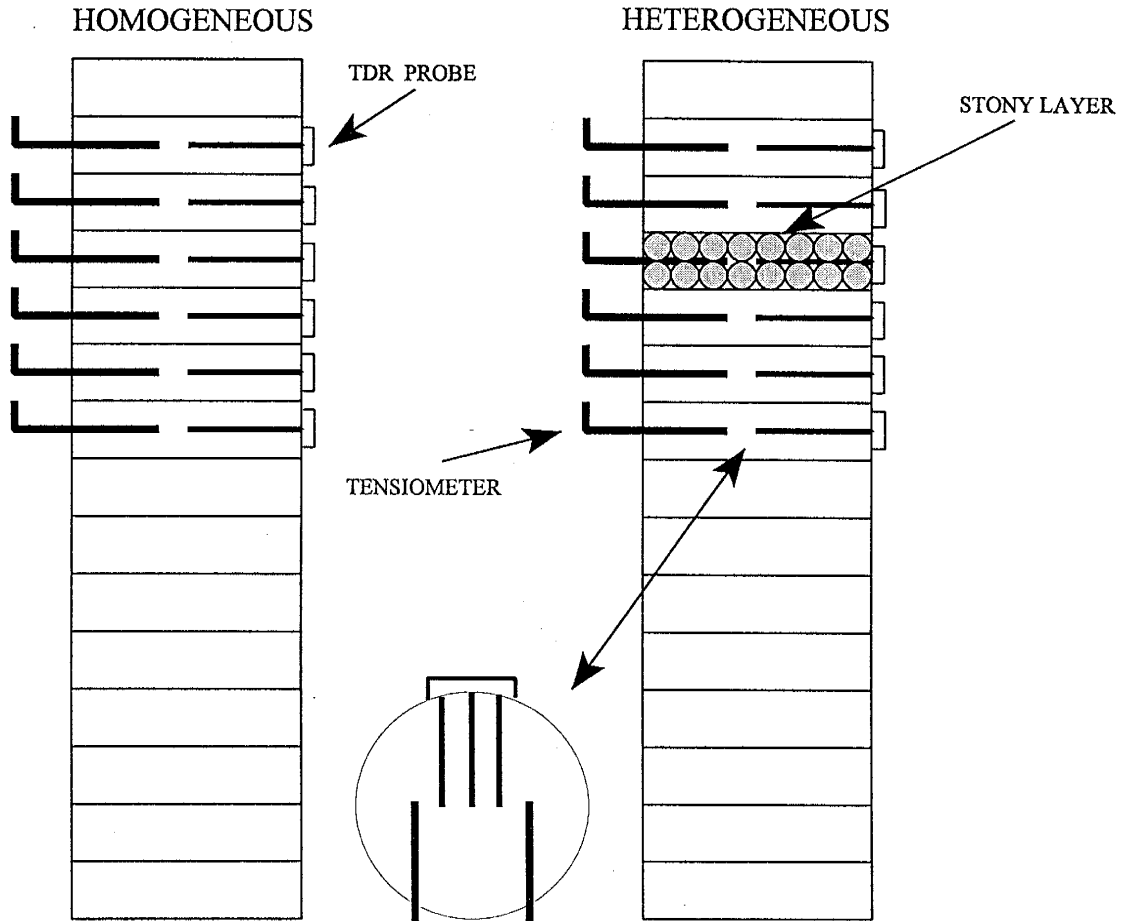


Figure 1. Schematic representation of soil columns illustrating homogeneous and heterogeneous profiles, along with instrument placement.

heterogeneous layered soil profile. Each soil profile was constructed using the following procedure. First, the field sand was placed in a bulk drying facility and air dried for a minimum of three weeks. Sand dried in this fashion typically has a residual water content of less than one tenth of one percent. The sand was then sieved to remove any foreign material that may have been introduced during the collection and drying process. The sieve was constructed from U.S. 20 size mesh, which was slightly larger than the largest particle diameter of the sand. The columns were then filled by pouring discrete quantities of the sand through a randomizer placed on top of the column. The randomizer consists of a series of screens and sieves that distribute the flow of sand uniformly over the entire area of the column. This promotes uniformity throughout the soil profile and helps to eliminate the formation of packing irregularities which may contribute to the development of preferential flow paths. Periodically during this process the sand in the column was settled by striking the top edge of the column with a rubber mallet. An equal number of strokes were administered at four points on the column edge separated by ninety degrees of arc. This helped to impart a uniform bulk density to the sand in the columns. Tensiometers were emplaced in the soil columns as they were filled with sand, at the designated elevations. This procedure was repeated until the column was completely filled.

Columns having layered profiles had an additional step included in the above procedure. When the sand reached the elevation of the base of the third measurement station, approximately 40 cm below ground surface, a layer of rubber spheres was placed in the column. These spheres were packed tightly in a uniform layer within the same horizon. A total of eighteen spheres fit in one layer and constitutes a volumetric percentage of 55% within the layer. The density of sphere packing was maintained in all of the heterogeneous experiments, only the thickness of this impeding layer was varied from experiment to experiment. When the thickness of the layer was increased the additional

balls were placed in the open macropore spaces between the balls of the layer immediately below in such a way as to maintain the density of packing and volumetric percentage. Sand was added concurrently with the spheres and settled as described above. Sand, when poured from a constant height, will tend to settle to a consistent bulk density. This is the basis of the sand-funnel technique used to determine the bulk density of soils insitu, (Klute 1986). This was the only additional step used in preparing the heterogeneous columns that differed from that of the homogeneous case.

TDR probes were inserted into the columns at each of the measurement stations after the profiles had been fully wetted. Approximately one hour before water application to the columns ceased, probes were inserted into the columns through access ports in the column walls. This was done so that the presence of the probes in the soil profile would not adversely effect the flow of water during the wetting phase of the experiment.

Applied Flux Conditions

A flux was induced in the soil columns by the application of water using a rain simulator. This device simulated rainfall by applying water through a series of small diameter needles, (Becton&Dickinson 20G1), producing droplets of water. These needles are attached to the bottom of a reservoir that has dimensions of 55 cm x 55 cm x 3 cm which is fed by a Master Flex peristaltic pump. The reservoir is constructed in such a way so that there is no change in storage during the experiment, so the metered pump reading is in fact the outflow rate over the application area of the reservoir. The reservoir is mounted in a steel frame that is actuated by two electric motors to move in two horizontal directions simultaneously. This helps to create a random pattern of water droplet application.

After preliminary experiments demonstrated the feasibility of this approach, a pumping rate of 0.13 cm/min was deemed the most appropriate for studying unsaturated flow in the field soil utilizing this experimental approach. Higher pumping rates induced saturated conditions in and near the last measurement station unnecessarily complicating the study. The period of water application lasted for approximately eight hours, typically overnight. Soil water tension was then checked at all of the measurement stations to ensure that a steady state condition had been achieved.

Instrumentation of Measurement Stations

Instrumentation of the soil columns consisted of six measurement stations being positioned at ten centimeter intervals starting at the second ring segment of the column. Each station consisted of two tensiometers and one TDR probe. All three devices were positioned in the center of the ten centimeter ring segments, coplanar with each other, (Figure 1). The TDR probe occupied a position between each of the two tensiometers extending into the column 15 cm. Tensiometers were placed on either side of the TDR probe, also extending into the column approximately 15 cm. Two tensiometers were used due to the size of the column, and the necessity of determining if the wetting of the column was uniform across its width. Tensiometers were constructed using materials that would not interfere with the functioning of the TDR. The materials consisted of nonconductive plastics and ceramic cups. As an added precaution they were placed outside of the volume of soil sampled by the TDR probe.

TDR Calibration

Three wire TDR probes were used in conjunction with a Tektronix 1502B cable tester to

collect measurements of volumetric water content. The probes consisted of three stainless steel wave guides 15 cm long with a separation distance of 3 cm. Data used in calibrating the TDR was plotted as measured apparent probe length versus volumetric water content for the field soil, and a linear regression fitted to the data. The equation of the line fitted to this data provided the calibration curve for the TDR in this field soil, where the x- coordinate is the measured apparent length of the TDR probe and the returned y intercept is the calculated volumetric water content.

TDR Sensitivity in Stony Soils

A crucial element in the study of unsaturated hydraulic conductivity is the accurate measurement of soil water content. When analyzing data using the instantaneous profile method the accuracy of this measurement is extremely critical because the timed rate of change of water content in the soil profile is an essential parameter of the analysis method. As such, how well the TDR performs in a soil with a high content of stones is of great interest. Drungil et al. (1989), state that the TDR can be used to determine water content in soils containing up to 50% coarse stone fragments, 0.5-1.25 cm in diameter, without a significant loss in accuracy. The effect of different soil types on TDR measurements was the focus of research conducted by Richardson et al. (1992), who stated that variable physical properties such as bulk density had no apparent effect on estimates of soil water content using the TDR technique. However, the work of Knight et al. (1994), Knight (1992), Baker & Lascano (1989) suggest that the TDR technique may be susceptible to perturbations occurring in the sample volume of the TDR probe. Due to this apparent conflict in the literature it was necessary to determine if the soil water content measurements collected during the instantaneous profile experiments were valid for the study of the heterogeneous profiles. The critical question as it pertains

to the heterogeneous profiles in this study is what is actually being measured in the stony layer by the TDR. Is the measurement a point measurement, i.e. the real matrix water content, or is it a larger volume averaged measurement including portions of the stones which are, in fact, dry in their interior? In order to properly address this question the sample volume of the three wire TDR probe used in this study was experimentally determined.

Experimental Determination of TDR Sample Volume

A series of known water content samples were created using the Sevilleta field soil by adding a known volume of water to a known volume of sand. The wetted sample, in sealed plastic bags, was then placed into an oven and warmed to a constant temperature of 38C° over a period of 24 hours. During this time the samples were mixed and rotated frequently in the sealed plastic bags. This process helped to distribute the water uniformly throughout the soil. The soil was then removed from the oven and allowed to cool to room temperature while it was again mixed and rotated in the plastic bag. Condensation on the bag surface was kept in contact with the soil to help minimize water loss and accurately reproduce the calculated water content. This uniformly wet soil was utilized in two experimental procedures to empirically determine the range of influence for the three wire TDR probe.

The first experimental procedure consisted of placing a mass of wet soil between two acrylic plates and recording the TDR measurements while the separation distance between the two plates was decreased in discrete intervals. The TDR probe was placed in the center of the soil mass bisecting the distance separating the two plates. Initial readings were taken at a separation distance of six centimeters, which is beyond the range of influence of the probe, (Baker & Lascano 1989, and

Topp and Davis 1985). This separation distance of six centimeters has been experimentally determined for a two wire probe arrangement to be the area of maximum dimension using 10% relative sensitivity as a threshold value, (Baker & Lascano 1989). Three wire probes have a significantly decreased spread of energy in the far field, (Knight et al. 1994), so this upper limit of six centimeters was chosen with confidence for the evaluation of the three wire probe. The separation distance was decreased until the layer of soil between the two acrylic plates was approximately ten to fifteen millimeters thick, which was significantly less than the thickness of the soil envelope placed around the probes in the stony profiles.

The second approach used in determining the field of influence around the probe wave guides was one focused on discerning the relative contribution of the outer two wave guides as compared to that of the central conductor. The three wire probe with one central conductor and two outer wave guides emulates a coaxial line and reduces the impedance mismatch between the wave guides and the coaxial transmission line. This results in a different spatial sensitivity function as well as a different energy distribution around the probe, as compared to a two wire probe. Most of the energy and most of the measurement weight is concentrated around the central wave guide of the three wire probe, (Zegelin et al. 1992). A series of small columns were packed with field soil prepared as discussed above. Each of the columns had a slightly smaller inner diameter than the one preceding it. The distribution of size ranges was 10 cm, 6.4 cm, and 3.1 cm. Initially this permits the probe to be completely inserted into the soil with the wave guides entirely surrounded by the medium. At the end point of column size only the central conductor is in the medium. In this way the relative sensitivity of the central conductor, as compared to the outer conductors, was determined for the three wire probe.

Using this approach the specific volume of soil being sampled by the TDR probe was experimentally determined for the instrument configuration used in this research. This empirical approach provided a quick and direct method of evaluating the volume of soil sampled by the TDR. Additionally, these methods may be extrapolated to any field soil or laboratory situation independent of scale without requiring a mathematically rigorous approach to estimate electromagnetic field distributions. If one chooses to estimate electric field densities or spatial weighting functions utilizing the theoretical approach, sufficient information regarding theory and examples be can found in Zegelin et al. (1992), Knight (1992) and Knight et al. (1994).

Soil Water Content

Volumetric water content was calculated for each of the column experiments from TDR readings using the above calibration curve. In the homogeneous soil profiles the calculated water content represents a volumetric water content due to the uniformity of the soil medium and distribution of soil moisture in it. Where volumetric water content is defined as

$$\theta_v = (\rho_b \theta_g) \div \rho_w \quad (1)$$

having ρ_b and ρ_w as the bulk density of dry soil and water respectively, and θ_g as gravimetric water content. Water content measurements made in stony layers represented only the matrix between the stones, based on the sample volume experiments, and had to be converted to volumetric water contents. This was done by multiplying the matrix water content by the volumetric percentage of stones in that layer. This volume averages the matrix water content converting it to a volumetric measurement. The equation used in this conversion is defined as

$$\theta_v = \theta_m \times (1 - V_{stones}) \quad (2)$$

having θ_m as the matrix water content and V_{stones} as the volume of stones in the layer one is working with.

Water Tension Measurements

Soil water tension measurements were collected using tensiometers. Tensiometers were constructed using small diameter ceramic cups and rigid plastic tubing. Ceramic cups were 1 inch long and 3/8 inch wide with a bubbling pressure of 800 mbars. The assembled tensiometers were L-shaped with each piece being approximately 15 cm long. Rubber septa were seated in the tubing opposite the cups using vacuum grease to prevent leakage. All tension measurements were corrected for the height of the water filled plastic tubing above the cup.

Instantaneous Profile Analysis

Data collected during the drainage phase of the column experiments consisted of soil water tension and soil water content. These data were analyzed using the instantaneous profile method developed by Hillel et al. (1972), which was derived from the work of Watson (1966). The method which Hillel derived is more properly suited to field situations where the water table is absent or too deep to effect soil moisture flow. This approach is more commonly applied to field situations and is considered a standard method in the field of soil physics. This was the method of analysis employed to analyze the raw data. The only variation from Hillel's method was the use of the TDR to measure soil water content instead of a neutron moisture probe. The following analytical procedure was employed in handling the raw data.

Soil water tension and soil water content measurements were collected simultaneously starting immediately after water application to the column ceased. During the first two hours of the post wetting time period readings were taken every fifteen minutes. Readings were then taken every thirty minutes for two more hours, at which point measurements were taken at intervals based on the rate of change in the observed data. The soil water content data were then plotted against time for each depth. The soil moisture flux for a given depth interval was calculated by fitting a function to the above curves using the mathematical curve fitting program Jandel Scientific Table Curve. This allowed the slope of the curve to be determined at any given point, which was then multiplied by its representative layer thickness to obtain the flux through each layer. Hydraulic head profiles were calculated and plotted for each measurement station as hydraulic head with depth through time. The hydraulic gradient was then determined for each station based on these plots. Hydraulic conductivity as a function of water content was then calculated by dividing the known flux through each layer, at the specific time intervals, by the corresponding hydraulic gradient values. These data were then plotted as hydraulic conductivity against volumetric water content and tension for analysis.

RESULTS AND DISCUSSION

Water Content Measurements

TDR Calibration

The results of calibrating the TDR to the field soil are presented as Figure 2. It shows data from one of three calibration experiments that were performed, it also shows the linear regression curve fitted to the experimental data.

A comparison of this calibration curve with the one developed by Topp et al. (1980) shows a high degree of correlation between the two when compared as dielectric constant versus volumetric water content. This comparison, presented as Figure 3, validates the method of calibration used, and suggests that this approach is a relatively easy way to calibrate the TDR to a particular field soil one is studying. The laboratory calibration curve specific to the Sevilleta field soil was used to determine water content in all of the column experiments.

Determination of TDR Sample Volume

Results of three parallel plate experiments are presented as Figures 4 through 6. Each of the experiments were conducted in identical fashion with the exception that the initial known water content varied from approximately 18% to 7%. The results suggest that the minimum thickness of soil, above and below the plane containing the wave guides, should be limited to 15 mm to maintain maximum confidence in water content measurements.

Figure 4 illustrates the effect on TDR measurements when the total soil layer thickness is

CALIBRATION CURVE FOR THE SEVILLETA FIELD SAND

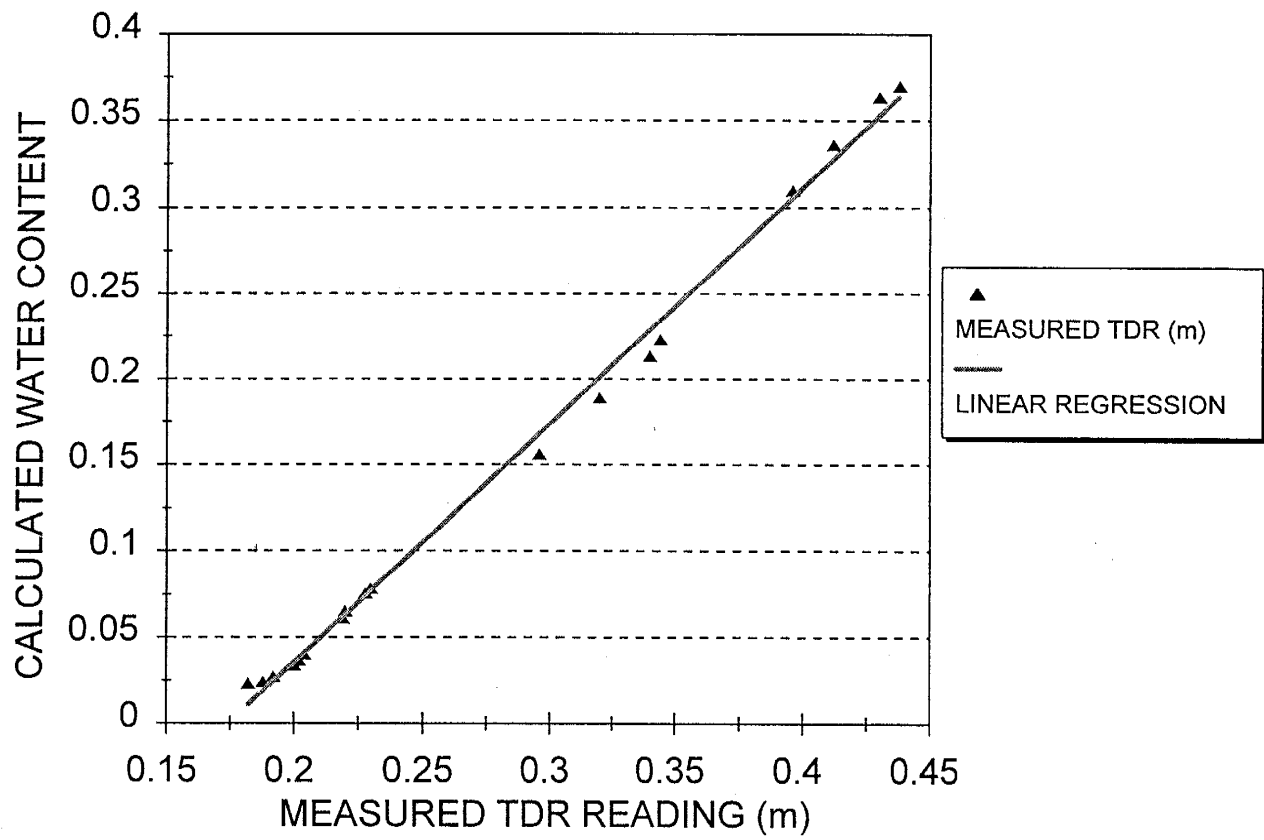


Figure 2. TDR calibration curve for the Sevilleta field sand.

THETA CALCULATED USING TOPP'S METHOD COMPARED TO LABORATORY CALIBRATION

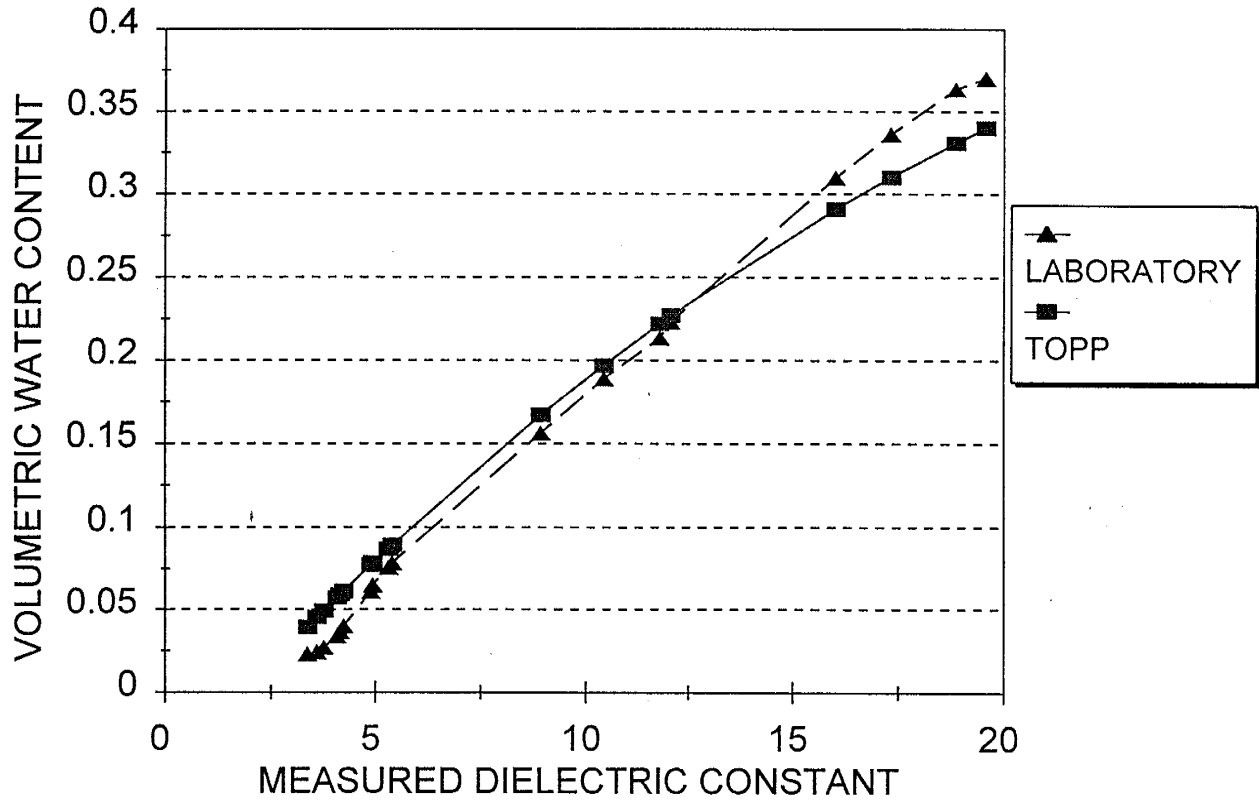


Figure 3. Comparison of estimated volumetric water content using the equation of Topp et al. (1980) as compared to that calculated using the laboratory calibration curve determined in this study.

INFLUENCE OF SOIL LAYER THICKNESS ON OBSERVED WATER CONTENT TRIAL 1

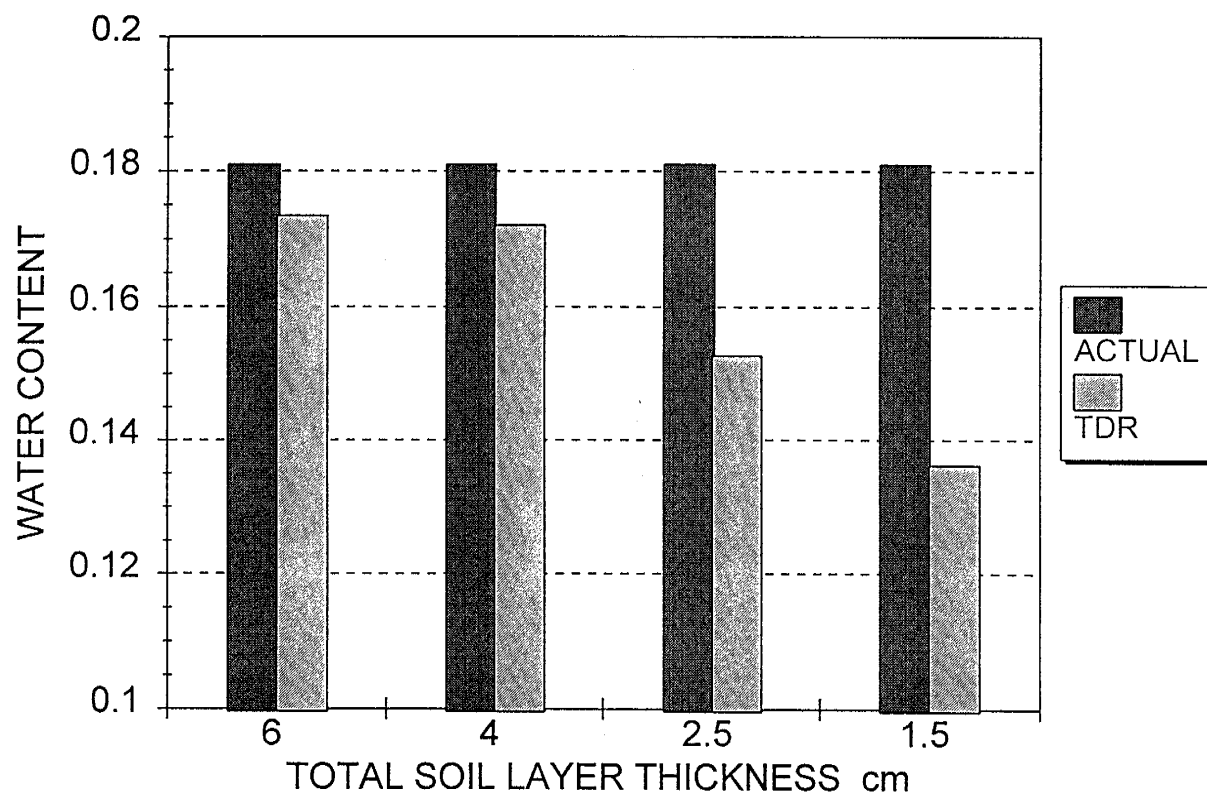


Figure 4. Influence of soil layer thickness on Time Domain Reflectometry measurement of soil water content. Trial 1, approximately eighteen percent water content.

decreased from 60 mm to 15 mm. For thicknesses of 60 mm and 40 mm the TDR readings were unaffected. At these distances the readings are within one percent of the calculated value of the soil as it was constructed. This is typical of nonautomated measurements made in the field or lab, and is near the resolution limit of the instrument in this configuration. Additionally, it should be noted that it is very difficult to prevent zero water loss during sample preparation and experimental manipulation. Decreasing the separation distance further to 25 mm and 15 mm the difference in measured water content as compared to the unaffected readings is an underestimation of approximately two and four percent. The presence of the dry acrylic plates within the sample volume of the TDR probe leads to an underestimation of the actual water content for the soil even though its physical properties were unchanged. With a volumetric water content of 18% this soil is moderately wet considering that the maximum field saturation ranges from 32% to 34%.

Figure 5 also illustrates the effect of decreasing soil layer thickness TDR measurements for a less wet soil. Again good agreement is observed between actual and measured water contents for separation distances of 60 mm and 40 mm, while significant measurement error begins to occur at a distance of 25 mm.

The soil in Figure 6 was the least wet of the soils tested in this fashion. It exhibits a similar trend of decreasing measurement accuracy with decreasing separation distance. Through a comparison of the three experiments it is clear that a homogeneous distribution of soil material within the field of influence surrounding the probe is essential to the accurate measurement of water content. Figure 7 is a plot of the error in volumetric water content between actual and measured values for each of the experiments at various separation distances. All of the measured readings in the range of 60 mm and 40 mm are within 1% of the actual values. At distances less than this the error in measured water

INFLUENCE OF SOIL LAYER THICKNESS ON OBSERVED WATER CONTENT TRIAL 2

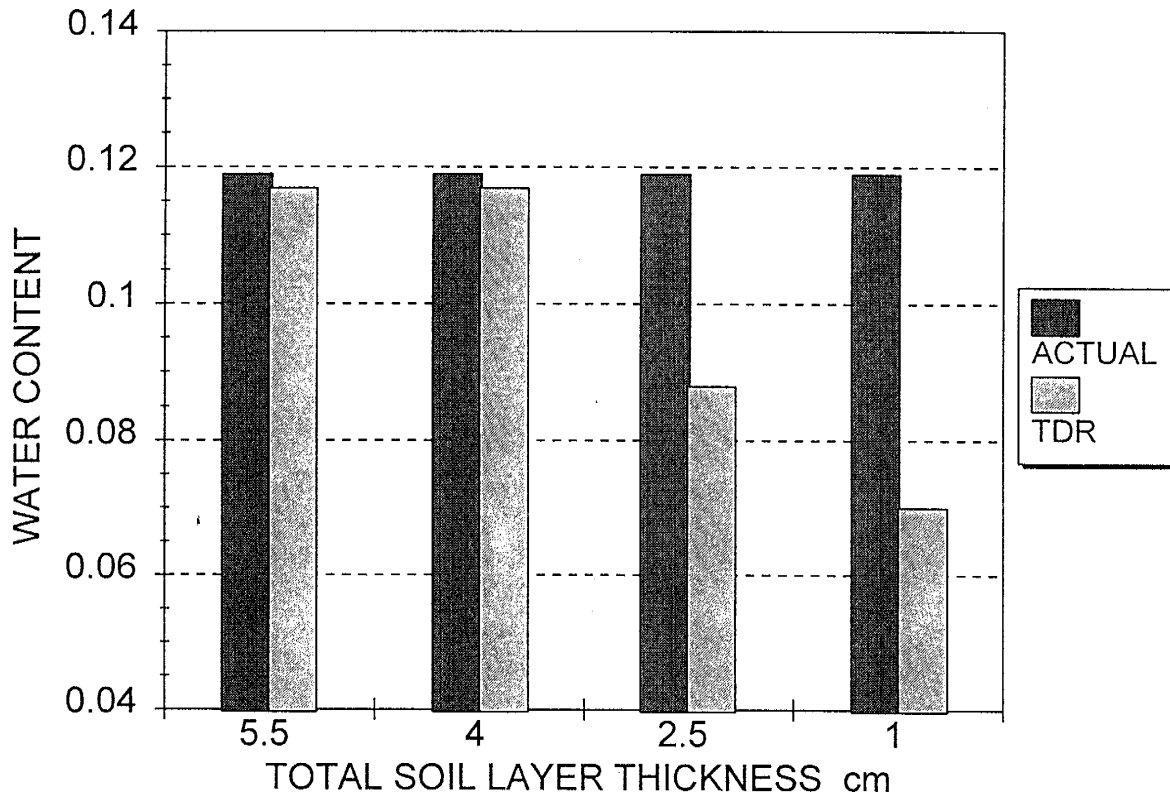


Figure 5. Influence of soil layer thickness on Time Domain Reflectometry measurement of soil water content. Trial 2, approximately twelve percent water content.

INFLUENCE OF SOIL LAYER THICKNESS ON OBSERVED WATER CONTENT TRIAL 3

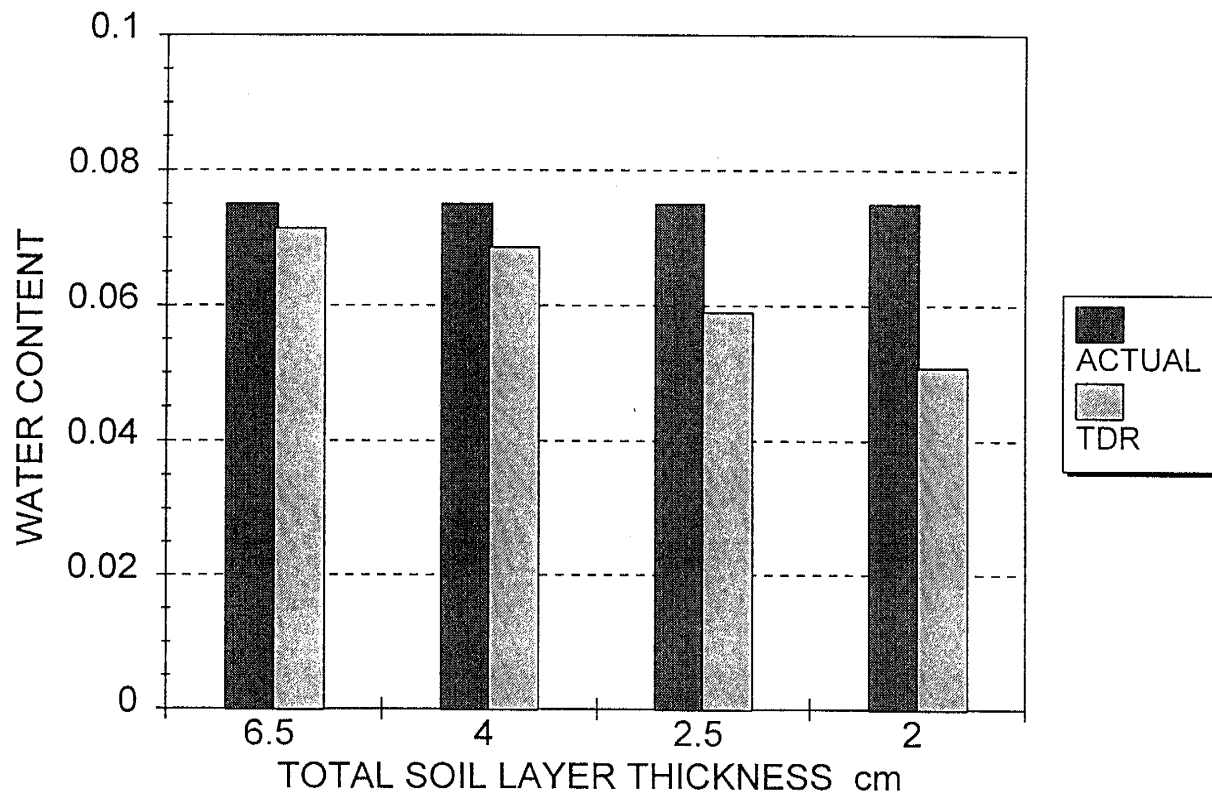


Figure 6. Influence of soil layer thickness on Time Domain Reflectometry measurement of soil water content. Trial 3, approximately eight percent water content.

DIFFERENCE IN WATER CONTENT AS A FUNCTION OF SOIL LAYER THICKNESS

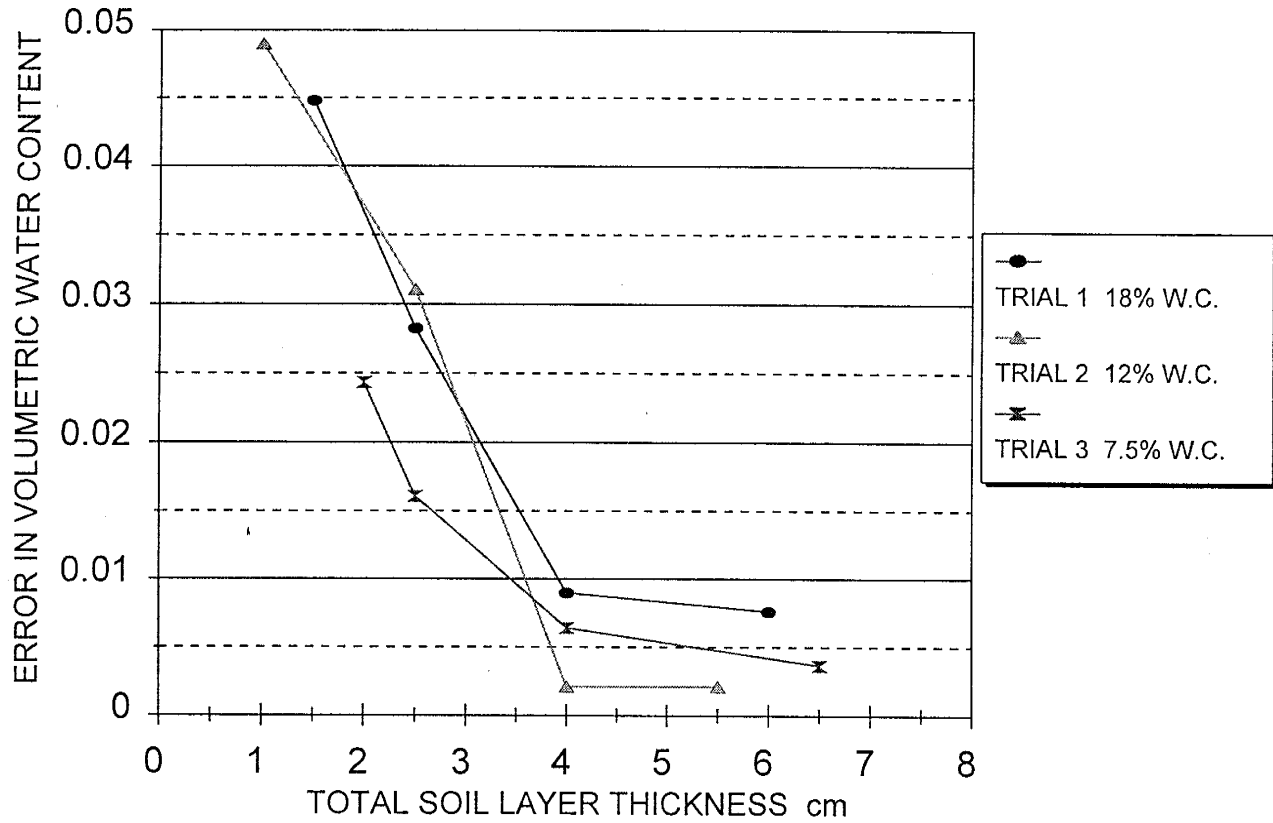


Figure 7. The observed error in volumetric water content between calculated and measured water content, as a function of soil layer thickness, for three soils having different initial water contents.

content increases and approaches a maximum of 5%. If the soil has a volumetric water content of 20% an error in the water content measurement of just 4% constitutes a relative error of 20% which is unacceptable for either field or laboratory work. This demonstrates that the maximum area of influence for the three pronged probe is relatively small, approximately 15 mm above and below the plane containing the wave guides of the probe. This result differs somewhat from that of Baker & Lascano (1989) where they describe an effective cross-sectional area containing most of the measurement sensitivity as 1000 mm² with dimensions of 20 mm x 65 mm, i.e. a 20 mm thick soil layer, for a probe having two wave guides 300 mm long separated by 50 mm. Some of the observed difference in sensitivity is due to the type of probe used, three wire verses two wire, while some difference may be attributed to the use of a soil matrix instead of water filled tubes oriented in an air matrix. In the absence of experimental data if one only examined the literature one would expect the area of influence to be significantly less then that measured in our tests.

Influence and Weighting of Wave Guides

The three wire probe, which emulates a coaxial conductor, strongly weights measurements as a function of position relative to the central conductor. These results are presented in Figures 8 and 9. Figure 8 shows a slightly higher observed water content than the actual value for the two cases in which the entire probe was surrounded by soil, still a difference of less than 1%. This is most likely the result of the way in which the columns were packed. The case where only the central conductor was in the soil the reading was 3% less than when the entire probe was is in the soil. A comparison of measured water content values indicates that the central conductor is responsible for approximately 65% of the total water content measurement. A measurement of 8.3% volumetric

SENSITIVITY OF PROBE WAVE GUIDES: REMOVAL OF OUTER CONDUCTOR INFLUENCE

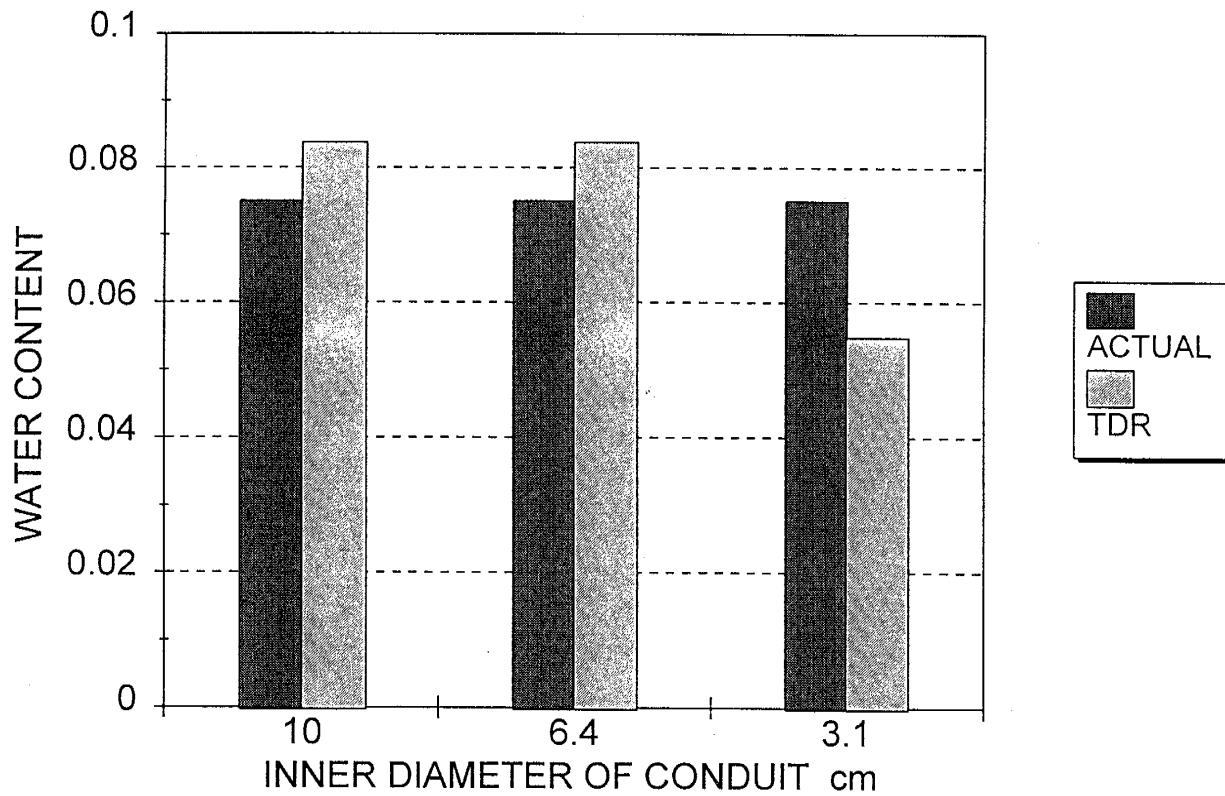


Figure 8. Sensitivity of Time Domain Reflectometry measurements to the removal of outer wave guide contribution.

ERROR IN VOLUMETRIC WATER CONTENT AS A FUNCTION OF OUTER CONDUCTOR

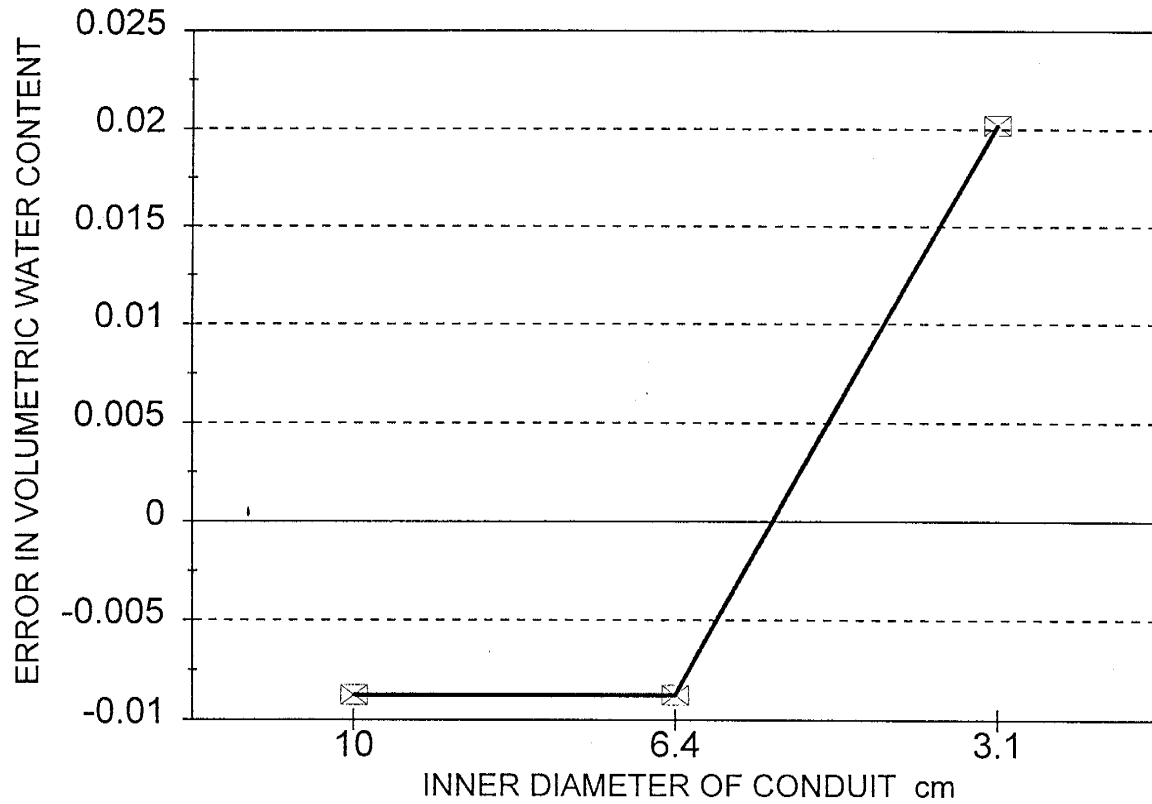


Figure 9. Error in the Time Domain Reflectometry measurement of a moist soil, removal of outer conductor contribution to the reading.

water content when the entire probe is in the soil and 5.4% when only the central conductor is in the soil. Zegelin et al. (1992) present several plots of electric field distributions for probes of various configurations. A comparison of these plots indicates that the electric field distribution lines are centered about the central conductor of the three wire probe with a greater density than in the two wire case. The above experimental finding is in agreement with this work and that performed by Knight (1992) which states that there is strong weighting of the TDR measurement of water content close to the central conductor of multiwire probes. This indicates that multiwire probes are better suited to measuring water content in stony soils, a result of the sample volume being confined to a smaller area about the central conductor.

Smaller "miniprobes" similar to those described by Sobczuk et al. (1992) would be close to ideal. Probes of this type sample a much smaller volume of soil so they are less likely to produce errors associated with the inclusion of dry stones in water content measurements. This reduction in sample volume returns a value more representative of a point than a larger volume averaged area, a distinct advantage when working in stony soils.

Correction for the Volume of Stones Present

Based on the experimental findings of the two approaches used to determine TDR sensitivity, and the available literature, it is reasonable to state that the TDR technique, when applied to stony soils, should be used with caution. It can not be used with impunity in all stony soil types. The design of the probe and its associated sample volume determine the type of water content being measured. Probes having large sample volumes, when used in stony soils, will return a measurement that is influenced by the matrix and the stones present in the soil profile. This measurement may or

may not be representative of the volumetric water content for the stony layer, a potential error when using the TDR technique in stony soils. However, probes of small design and associated small sample volume will typically return values that are more representative of the matrix water content alone. These "real" matrix water contents must then be converted to volumetric water contents for use in analyzing data. The probes used in this study have such a small sample volume that the water content measurements collected were representative of the matrix between the stones. As a result of this the matrix water contents had to be converted to volumetric water contents in order to properly analyze the data.

The true volumetric water content for the heterogeneous profiles was calculated using equation two. This converts the real matrix water content to a volumetric water content, through volume averaging, for the stony layer. The spatial distribution of stones in the profile and the volumetric percentage of stones present are critical factors used for correcting water content measurements in stony soils. One must know if the water content values collected using the TDR are volumetric water contents, representing an entire horizon or true matrix water contents representing only that portion of the horizon filled with matrix. An inability to determine type of water content measurement being recorded, and not correcting for the volume of stones present in the layer, can lead to significant error in the calculation of unsaturated hydraulic conductivity, due to its exponential nature.

The construction of the heterogeneous soil profiles allowed an envelope of soil matrix to be placed around the TDR probes and tensiometers so that stones were not in direct contact with the instruments. This combined with the above empirical findings dictates that the soil water tension and water content readings represent true or "real" matrix values. When referring to water content measurements in the remainder of the paper it will be clearly stated as being the real matrix water

content or volumetric water content.

Hydraulic Properties

Soil Water Retention

The observed and fitted soil water retention curve for the homogeneous profile is presented as Figure 10. It should be recognized that these results are for a transient system. Unlike soil water retention relationships obtained by hanging column methods, which by their very nature are static equilibrium measurements, the data presented in this figure document a dynamic relation between soil water tension and volumetric water content. True equilibrium conditions may not have been achieved during the experiment due to the transient nature of the analysis method. The fitted curve was developed by inputting $\Psi(\theta)$ and $K(\theta)$ data into the unsaturated soils model RETC, utilizing a simultaneous fit along with the *van Genuchten - Mualem* models for analysis. This fitted curve was then used as a baseline condition for the purpose of comparing the heterogeneous and homogeneous soil profiles.

Figure 11 presents the results for a single impeding layer 6.5 cm thick with a volume percent of stones of 55% within the layer. A bifurcation in the previously observed data trend results. The bifurcation seen in the data is explained as being the result of differential water contents existing in the soil profile at the beginning of the experiment due to the impeding layer. This differentiation of water content is responsible for initiating a unique scanning curve for each layer as desorption takes place within that layer. Scanning behavior and hysteresis effects have been observed in field soils in numerous studies, Sobczuk et al. (1992), Dane and Wierenga (1975), Poulouvasilis and Tzimas

SOIL WATER RETENTION

HOMOGENEOUS SOIL

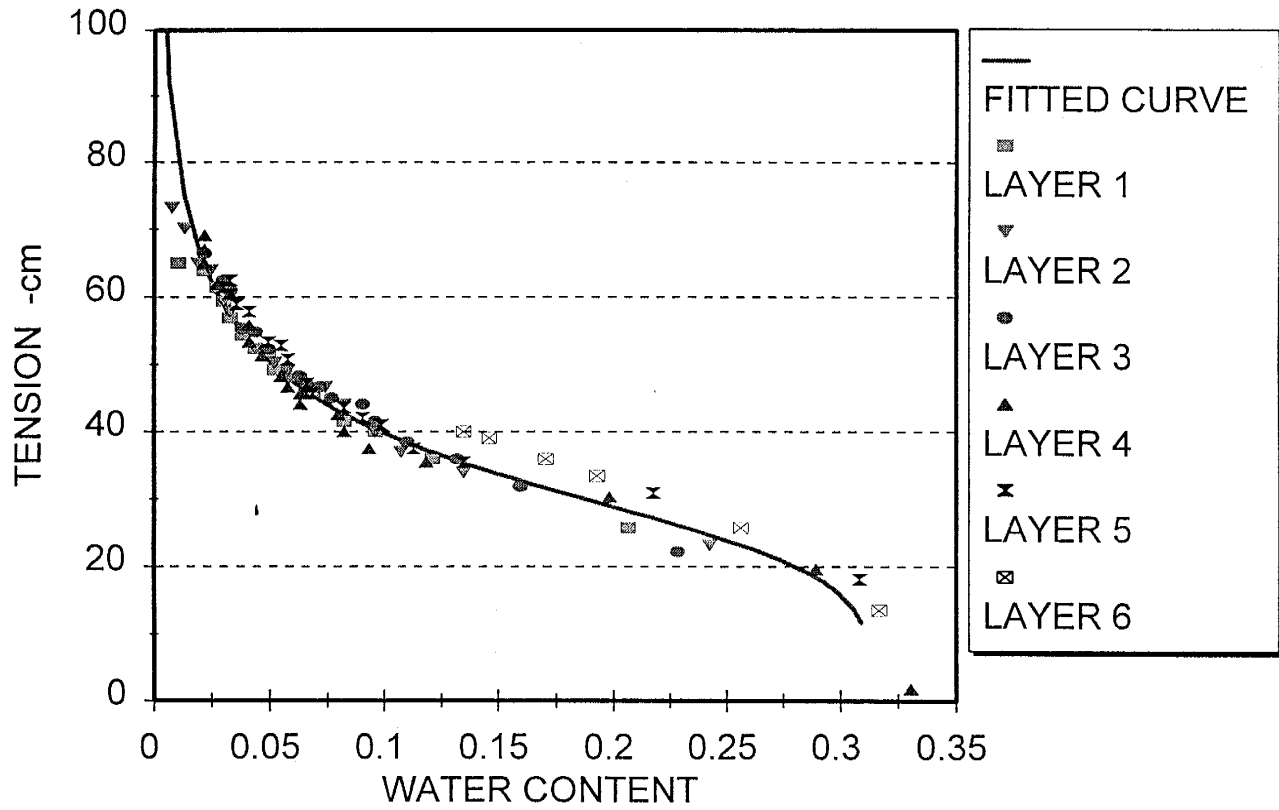


Figure 10. Soil water retention curve for the Sevilleta field soil, homogeneous soil profile. RETC curve fit utilizing the models of *van Genuchten* and *Mualem*.

SOIL WATER RETENTION 6.5 CENTIMETER LAYER

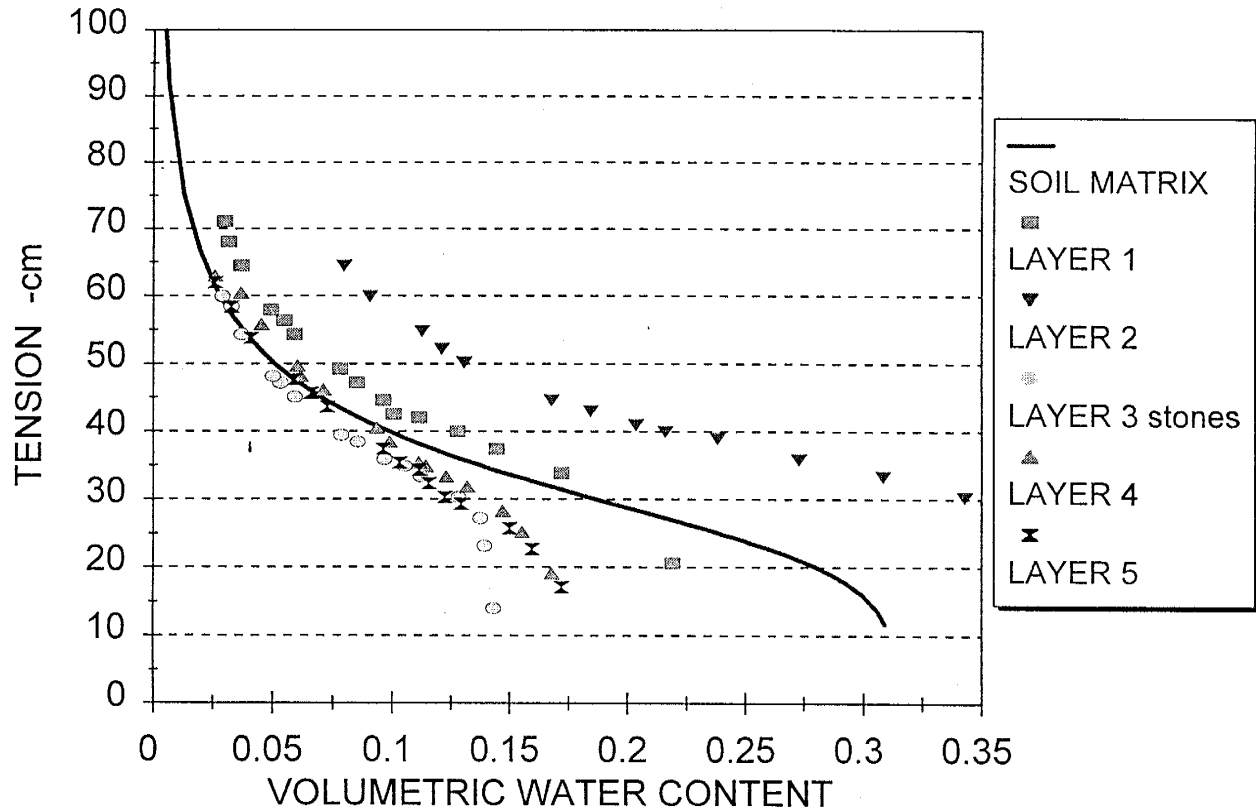


Figure 11. Soil water retention curve for the Sevilleta field soil, heterogeneous soil profile. Impeding stone layer 6.5 centimeters thick in layer three. Volumetric water content, (θ_v), corrected for the volume of stones present.

(1974).

The water content of the stone layer was corrected for the volume of stones present using equation 2. Layer 2 demonstrates a markedly different scanning behavior due to the inclusion of stones in the soil profile. This is the result of that layer experiencing, to a greater extent than the other layers, the effects of drainage for a water content near saturation. It naturally more closely approximates the true drainage scanning curve, with the associated higher tensions, for the field soil than the less wet layers above and below it. Layers 4 & 5 behave similarly, a result of their initial water contents being nearly identical. As such, they both follow scanning curves that closely approximate one another. It is interesting to note that even though both of these matrix layers are below the stony layer their soil water retention character is effected by its presence.

The results of the 10 centimeter impeding stone layer are presented as Figure 12. The data is again bifurcated as compared to the homogeneous case. All layers approximate a similar trend with the exception of layer 2, which is immediately above the stone layer. Layer 2 is again distinctly offset from the layers above or below it. It is clear that the impeding stone layer, i.e. layer 3, is exerting an influence on the soil horizon ten centimeters above it, affecting the unsaturated flow of water in this region. Again layers 4 & 5 follow nearly identical scanning curves as desorption occurs in the profile while layer 3 behaves distinctly different than the matrix layers.

Figure 13 presents data for the 16 centimeter thick stone layer. The data presented in this plot again shows bifurcated trends in the soil water retention curve similar to the previous two heterogeneous soil profiles. The soil water retention character of the soil is so effected by the presence of the impeding stone layer that the layers composed only of matrix no longer approximate the soil water characteristic of the homogeneous profile. This is well evidenced by the fact that

SOIL WATER RETENTION 10 CENTIMETER STONE LAYER

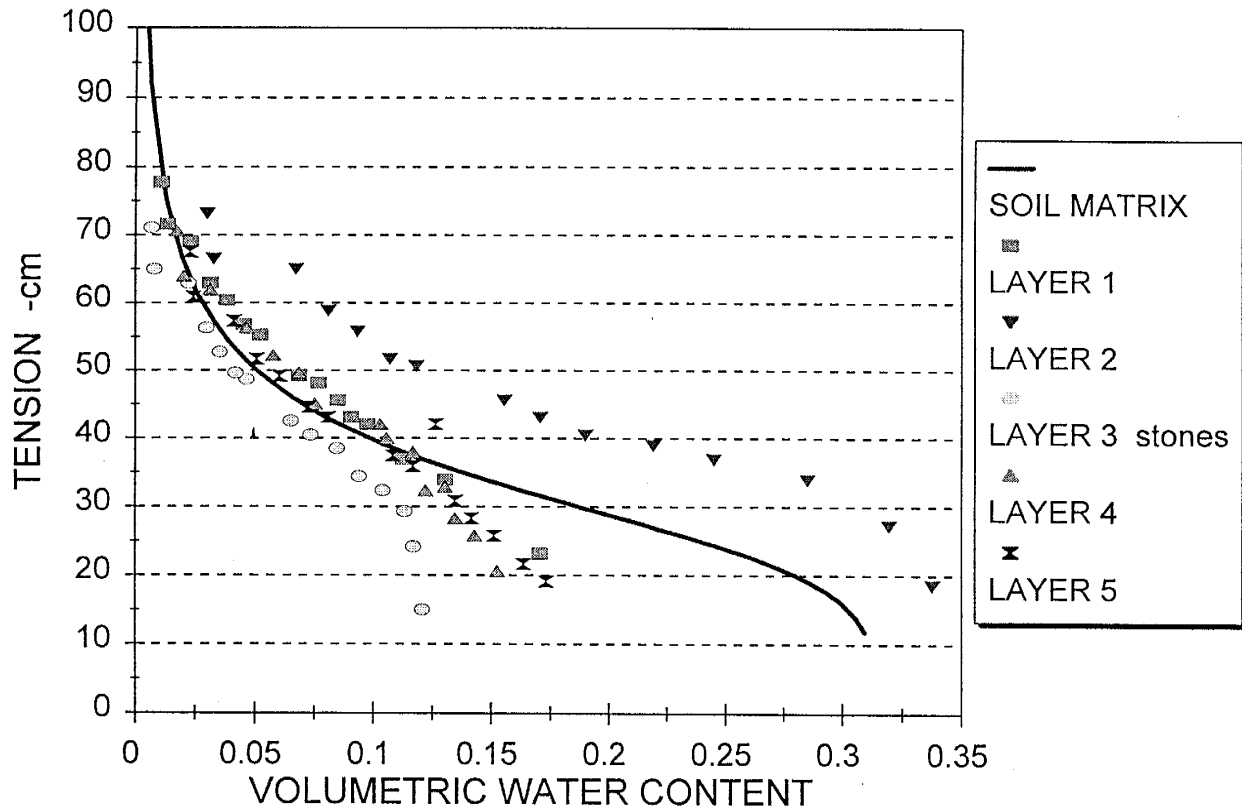


Figure 12. Soil water retention curve for the Sevilleta filed soil, heterogeneous soil profile. Impeding stone layer 10 centimeters thick in layer 3. Volumetric water content, (θ_v), corrected for the volume of stones present.

SOIL WATER RETENTION 16 CENTIMETER STONE LAYER

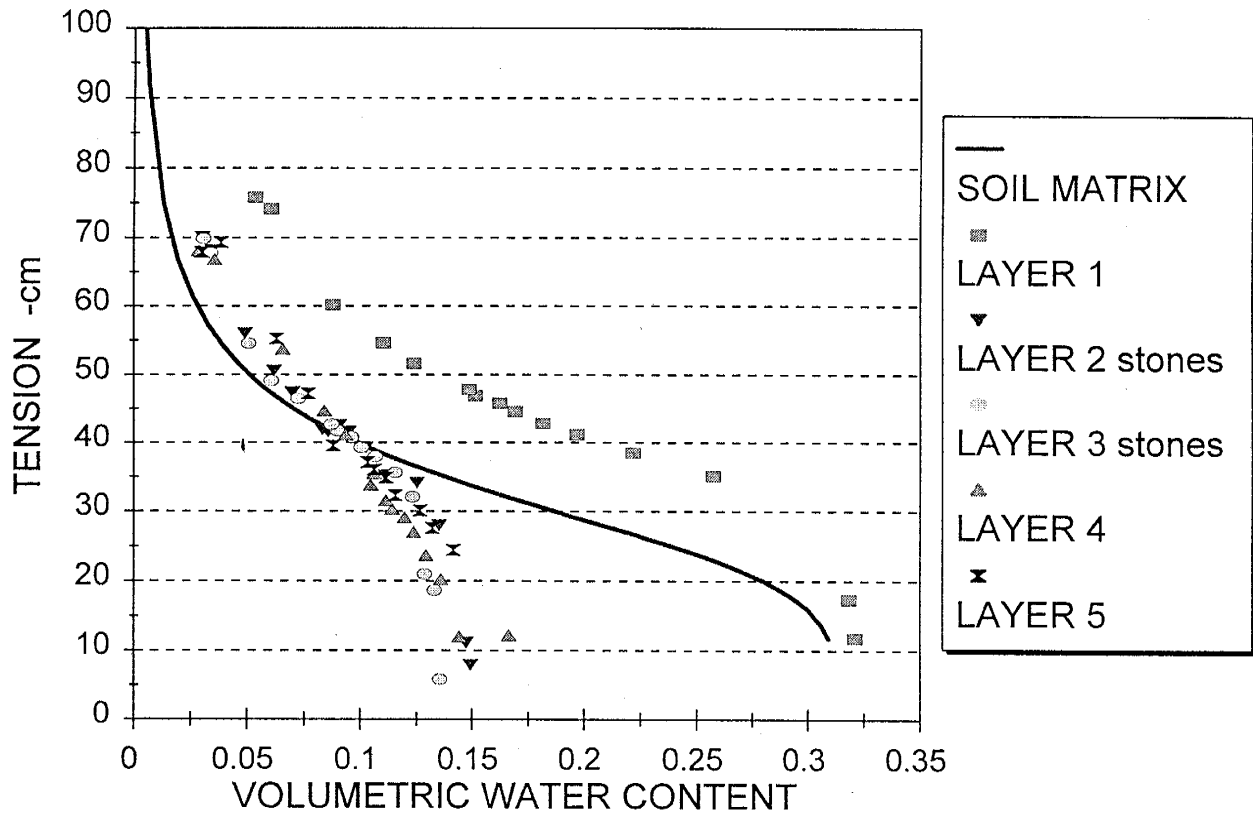


Figure 13. Soil water retention curve for the Sevilleta field soil, heterogeneous soil profile. Impeding stone layer 16 centimeters thick in layers two and three. Volumetric water content, (θ_v), corrected for the volume of stones present.

layers 4 & 5 now behave different from one another and no longer approximate the fitted curve for the soil matrix. The presence of such hysteric behavior in a layered soil with varying water contents is quite reasonable, if not somewhat unexpected.

Conductivity and Tension Relationships

In an effort to further quantify changes in the unsaturated properties of layered stony soils the unsaturated hydraulic conductivity was plotted against tension for both the homogeneous and heterogeneous cases. The graphs illustrate how the conductivity-tension relationship is influenced by the scanning behavior first observed in the soil water retention plots. Trends observed in the $K(h)$ data correspond to increases and decreases in the soil water tension due to hysteresis. These data are presented as Figures 14 through 17. Figure 14 illustrates the relation between hydraulic conductivity and tension for the homogeneous soil profile. The fitted curve was the result of using RETC to fit the data as described above.

Figure 15 presents the relationship between conductivity and soil water tension for an impeding stone layer of 6.5 centimeters. Data from layers 1, 3, 5 are used to assess the behavior of the soil profile. Layers 3 and 4 are not presented for reasons of clarity, while the layers shown correspond to positions above, within, and below the impeding stone layer. The behavior of each of these layers is distinctly different from those in the heterogeneous profile. Layer 1 indicates that at low water contents, and similar conductivities, tensions are higher when the stony layer is present. At higher volumetric water contents the observed tensions are lower when the stone layer is present. Measured tensions in the stone layer and layer below it are consistently lower than for the homogeneous case.

CONDUCTIVITY vs SOIL WATER TENSION HOMOGENEOUS SOIL

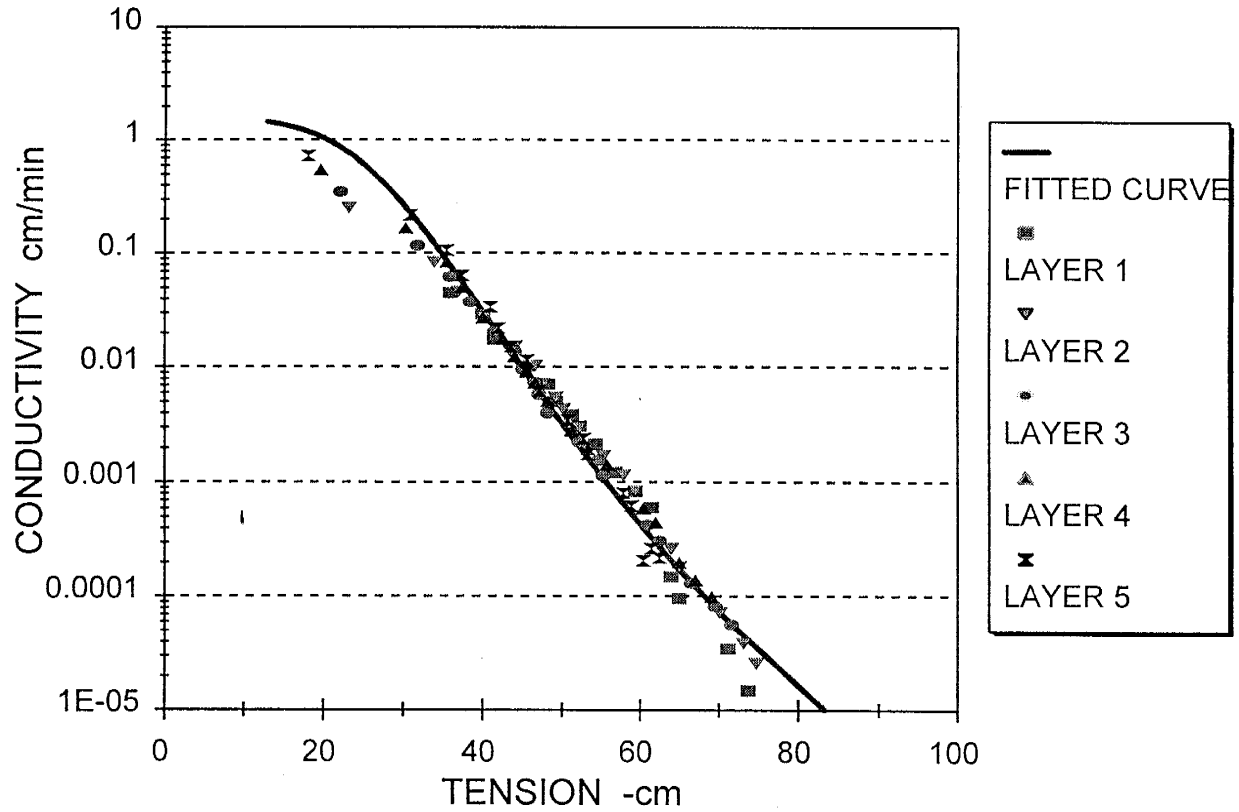


Figure 14. Unsaturated hydraulic conductivity verses tension, homogeneous soil profile. RETC fitted curve utilizing the models of *van Genuchten* and *Mualem*.

CONDUCTIVITY vs SOIL WATER TENSION

6.5 CENTIMETER STONE LAYER

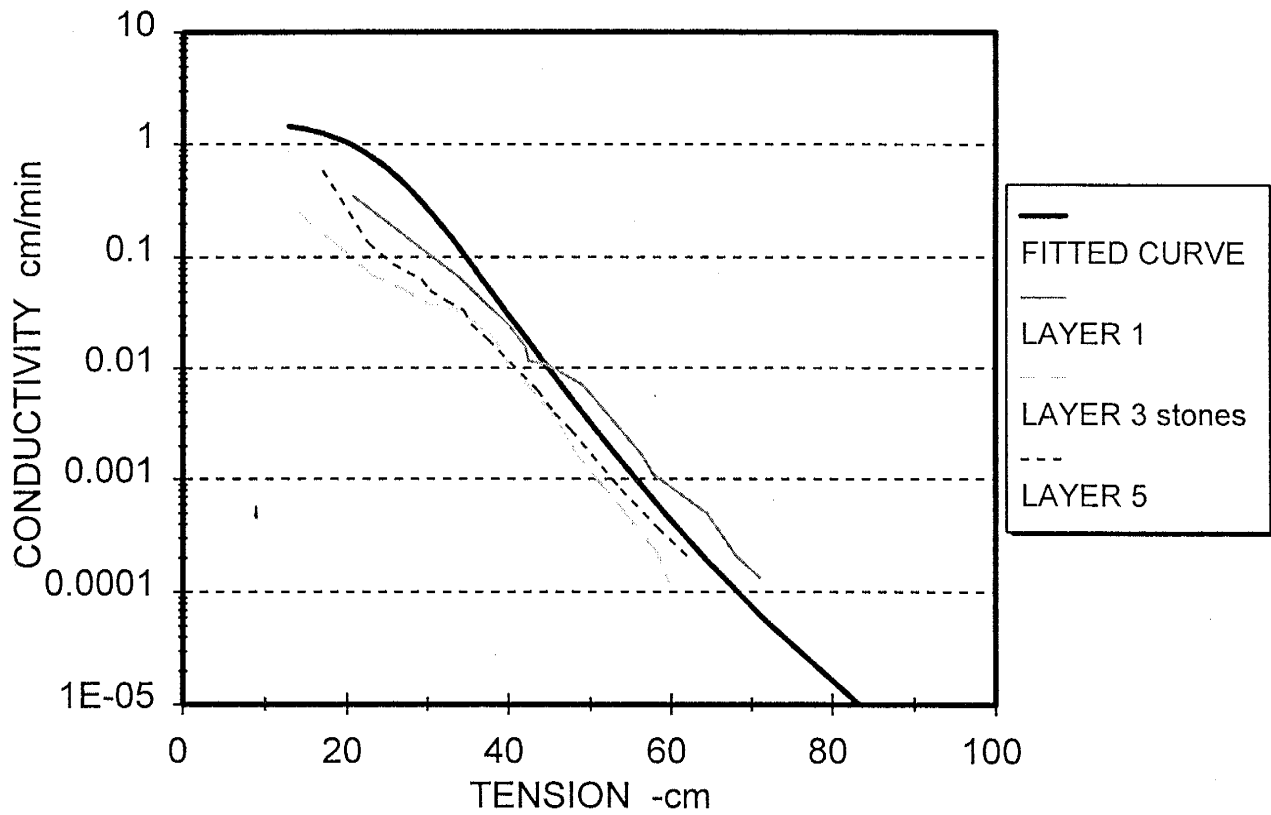


Figure 15. Unsaturated hydraulic conductivity verses tension. Heterogeneous soil profile, 6.5 centimeter stone layer.

When examined in conjunction with the data in Figure 11 this behavior can be explained as resulting from the different scanning curves initiated in each of the layers due to different initial water contents. Therefore, one would expect to see similar or identical hydraulic conductivity values at different soil water tensions, depending on the specific scanning curve followed during desorption. And this is, in fact, what is observed in the data.

Figure 16 is for the 10 centimeter impeding stone layer. Again all three layers behave differently than in the homogeneous case. Tensions for low volumetric water contents are higher than predicted for layers 1&3, while tensions at high water contents are lower than predicted for all layers. When compared to the soil water retention data for this soil profile the behavior is similar for each of the layers, (Figure 12). In each case, i.e. for each layer, the values of tension begin at values below the fitted curve, cross it at some critical threshold value, and end up at values higher than predicted. Clearly similar conductivities are occurring at different tensions due to hysteresis effects.

Data for the 16 centimeter stone layer, Figure 17, also exhibits similar hysteric behavior based on the observed tensions. Increasing the thickness of the impeding layer has more strongly effected the upper most layer than either the stone layer or the layer below it. This is the result of the top layer having a significantly higher initial volumetric water content than either of the other layers. This strongly influences the starting point of the scanning curve for this layer, and its desorption character during the remainder of the experiment. As in the previous heterogeneous profiles the $K(h)$ curve for the stony layer is below the matrix layer immediately above it in the soil profile, and tends to run parallel to it. This experimental result was observed in each of the heterogeneous profiles, and is consistent with behavior predicted by Bower and Rice (1984) for stony soils using a theoretical approach based on stone geometry and the matrix $K(\Psi)$ characteristic. The observed experimental

CONDUCTIVITY vs SOIL WATER TENSION

10 CENTIMETER STONE LAYER

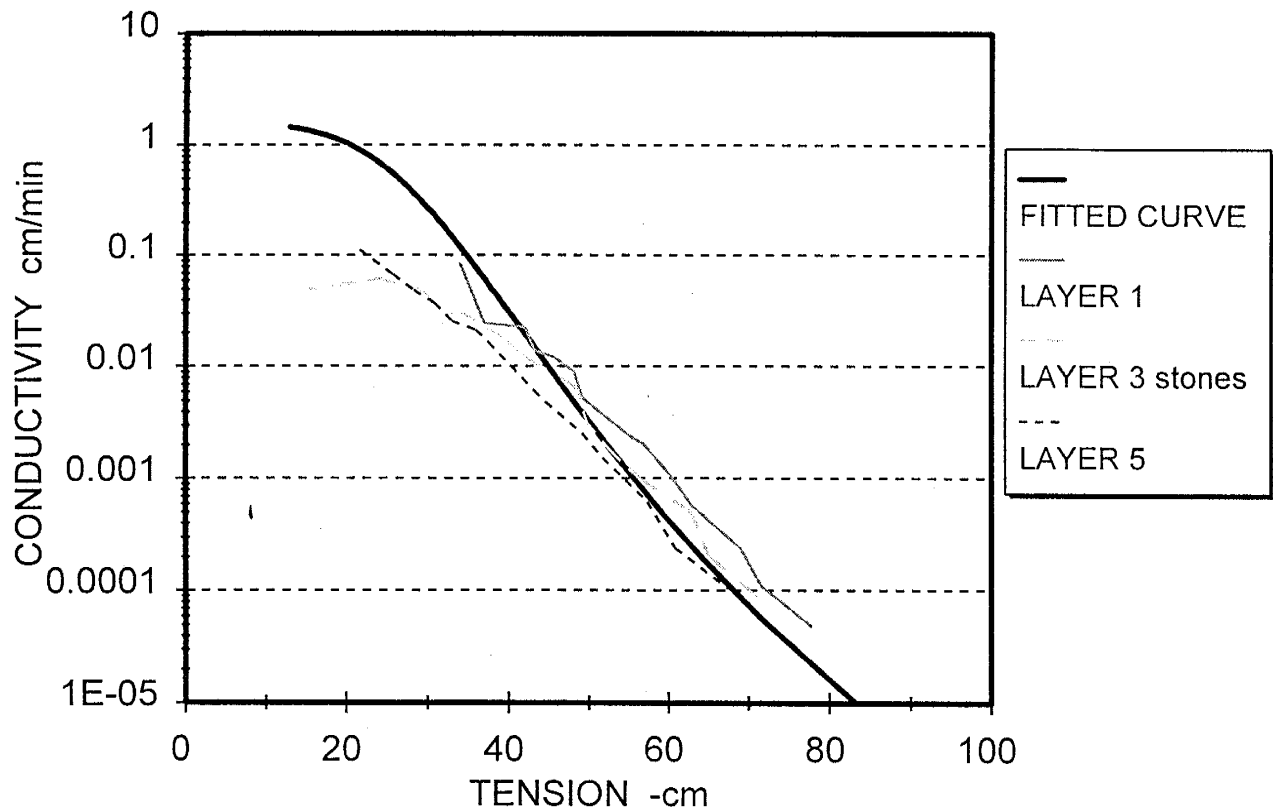


Figure 16. Unsaturated hydraulic conductivity verses tension. Heterogeneous soil profile, 10 centimeter stone layer.

CONDUCTIVITY vs SOIL WATER TENSION

16 CENTIMETER STONE LAYER

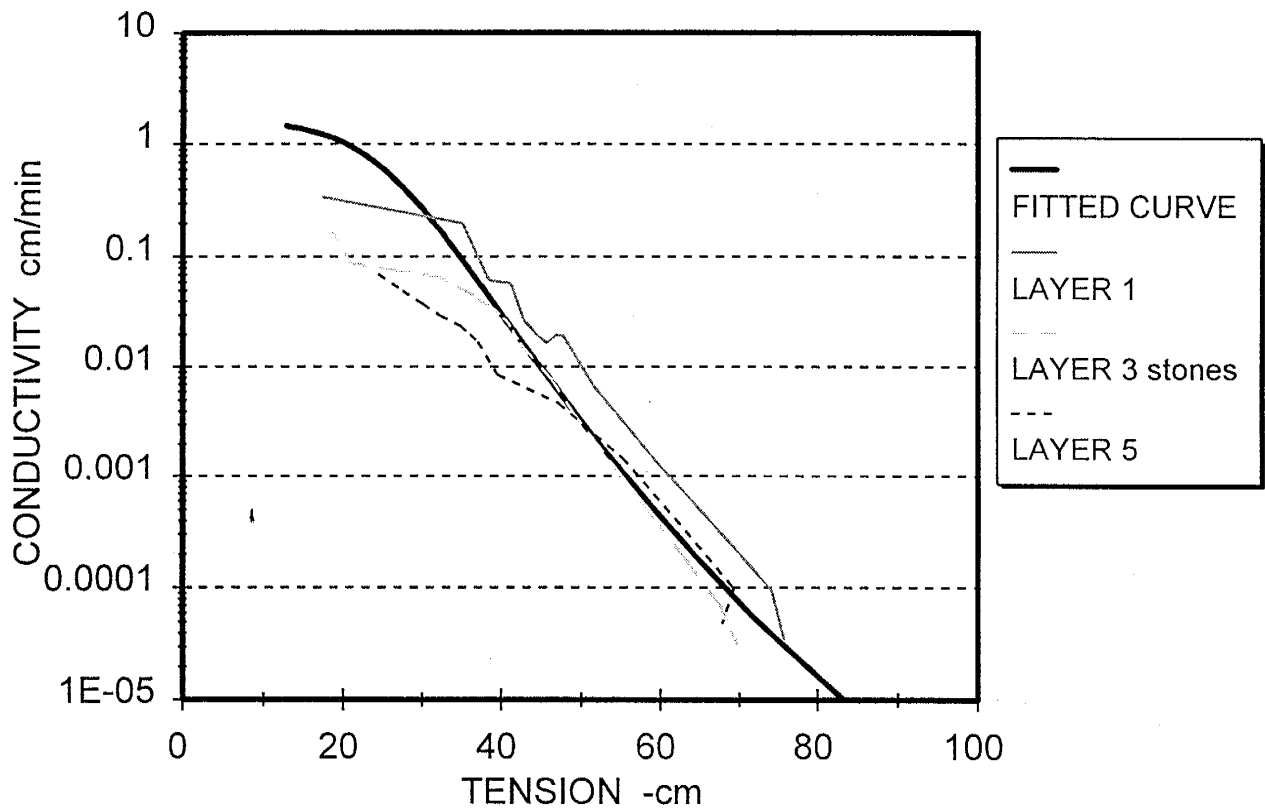


Figure 17. Unsaturated hydraulic conductivity verses tension. Heterogeneous soil profile, 16 centimeter stone layer.

data validates the theoretical approach of Bower and Rice (1984) as well as the results obtained in the laboratory experiments. Further strengthening the argument for hysteresis effects in stony layered soils.

Hysteresis effects on the relationship between hydraulic conductivity and suction have been observed in nonlayered soil profiles by Sobczuk et al. (1992), Poulouvassilis and Tzimas (1974), Vachaud and Thony (1971), and in layered profiles by Dane and Wierenga (1975). The observed results indicate that the presence of an impeding stone layer increases the likelihood of such hysteric behavior occurring.

Hydraulic Conductivity Water Content Relationships

Hydraulic conductivity as a function of volumetric water content is presented in Figures 18 through 22. Figure 18 shows the relationship between unsaturated conductivity and volumetric water content for the homogeneous soil profile.

Figure 19 presents data for an impeding stone layer of 6.5 centimeters. All layers approximate the trend of the homogeneous soil profile with the exception of layer 2, the layer immediately above the stone layer. The stone layer shows a trend toward a slightly higher conductivity at a similar volumetric water content. Soil water retention data for layer three with a 6.5 centimeter impeding stone layer shows that at low volumetric water contents measured tensions approximate the fitted curve for the homogeneous profile, while at higher volumetric water contents the tension is somewhat less than the fitted values. This trend is also observed in the hydraulic conductivity data where at lower volumetric water contents the data approximates the fitted curve and at higher volumetric water contents the conductivity is slightly greater than the fitted values. This is the result

UNSATURATED CONDUCTIVITY HOMOGENEOUS SOIL

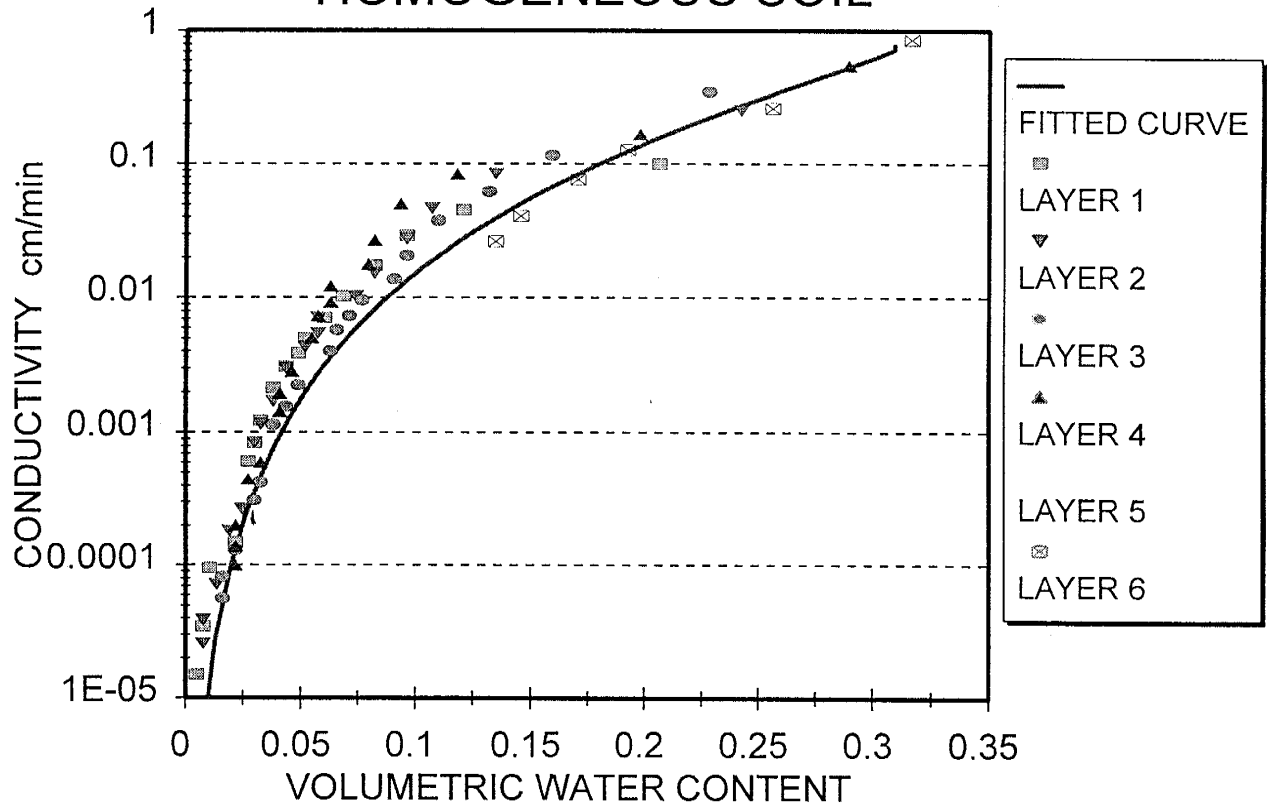


Figure 18. Unsaturated hydraulic conductivity versus volumetric water content, homogeneous soil profile. RETC curve fit utilizing the models of *van Genuchten* and *Mualem*.

UNSATURATED CONDUCTIVITY 6.5 CENTIMETER STONE LAYER

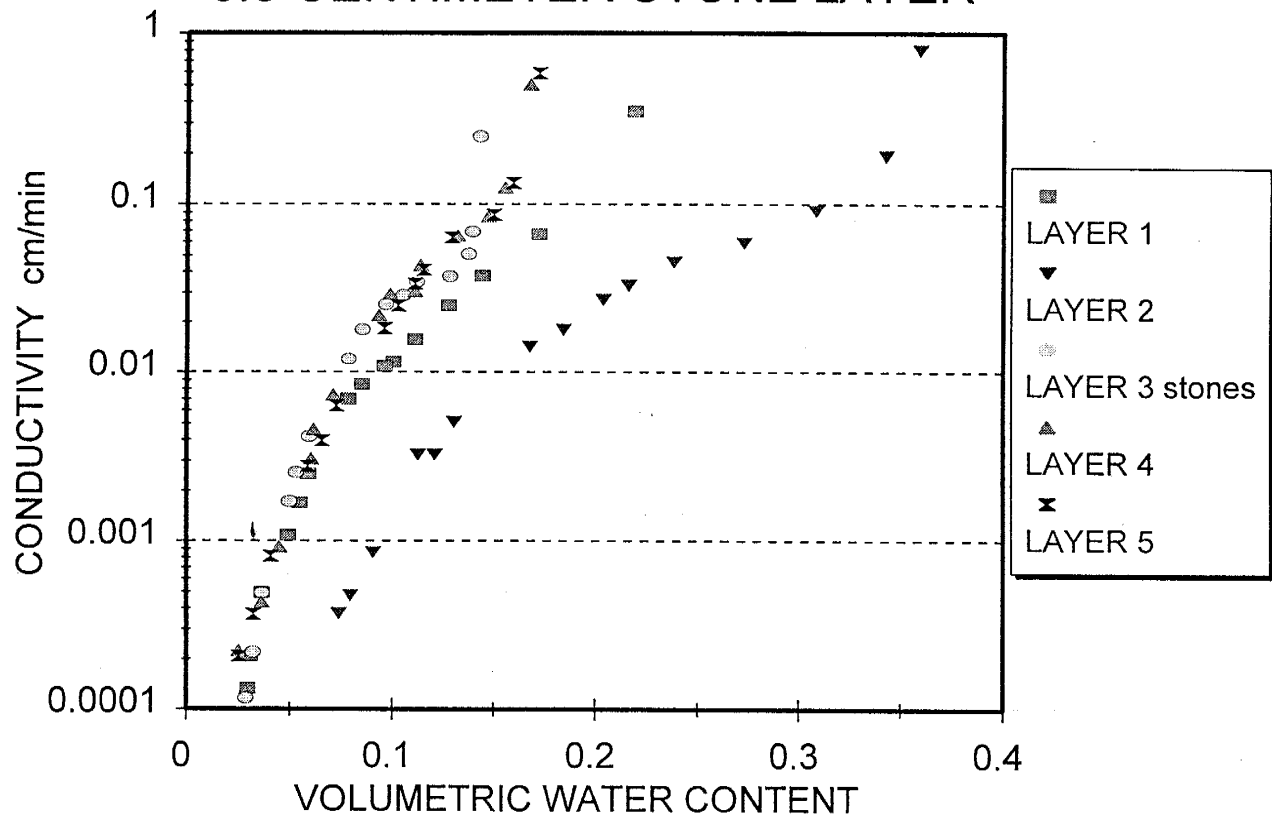


Figure 19. Unsaturated hydraulic conductivity versus volumetric water content, (θ_v), heterogeneous soil profile. Water content corrected for the volume of stones present.

of a hysteresis effect where lower tensions are occurring at similar water contents permitting higher conductivity values to exist in the soil. The behavior of layer 2 is interesting because it suggests that a low conductivity layer is present in the soil when, in fact, none exists. An examination of the data provided an explanation for this behavior. A comparison of the calculated flux and gradient for both the homogeneous and heterogeneous profiles revealed that the heterogeneous profiles had lower fluxes at higher water contents, along with gradients that were overestimated. This produces conductivities that are unreasonably low for such high water contents. A lower flux value divided by an overestimated gradient value yields a lower unsaturated hydraulic conductivity value for a given water content. This result when examined in conjunction with the lower fluxes that are occurring at substantially higher water contents explains the behavior of layer 2. The observed fluxes are unreasonable given the high volumetric water content for the layer, a result of the stone layer impeding the flow of water out of the profile. The high gradient values are a result of the analysis method being unable to properly estimate the gradient in the region immediately above the stone layer. The linear approximation of the hydraulic gradient between two measurement depths in the instantaneous profile method is not capable of accurately predicting the nonlinear gradients occurring in the profile, even though the data points are only 10 cm apart. The improper estimation of the hydraulic gradient in the region of the stone layer combined with the reduced flux at high water content suggests that a low conductivity layer is present at a location in the soil where none exists. This "pseudo" layer appears on each of the remaining two heterogeneous conductivity plots. A simulation was run for the heterogeneous profiles using Chain _ 2D, an unsaturated flow model, to examine hydraulic gradients in the region of the stone layer. The result illustrates the nonlinear nature of the gradient in the region of the stone layer, and is presented as Figure 20.

SIMULATION OF HEAD PROFILE WITH DEPTH STONE LAYER AT 30 - 40 CM INTERVAL

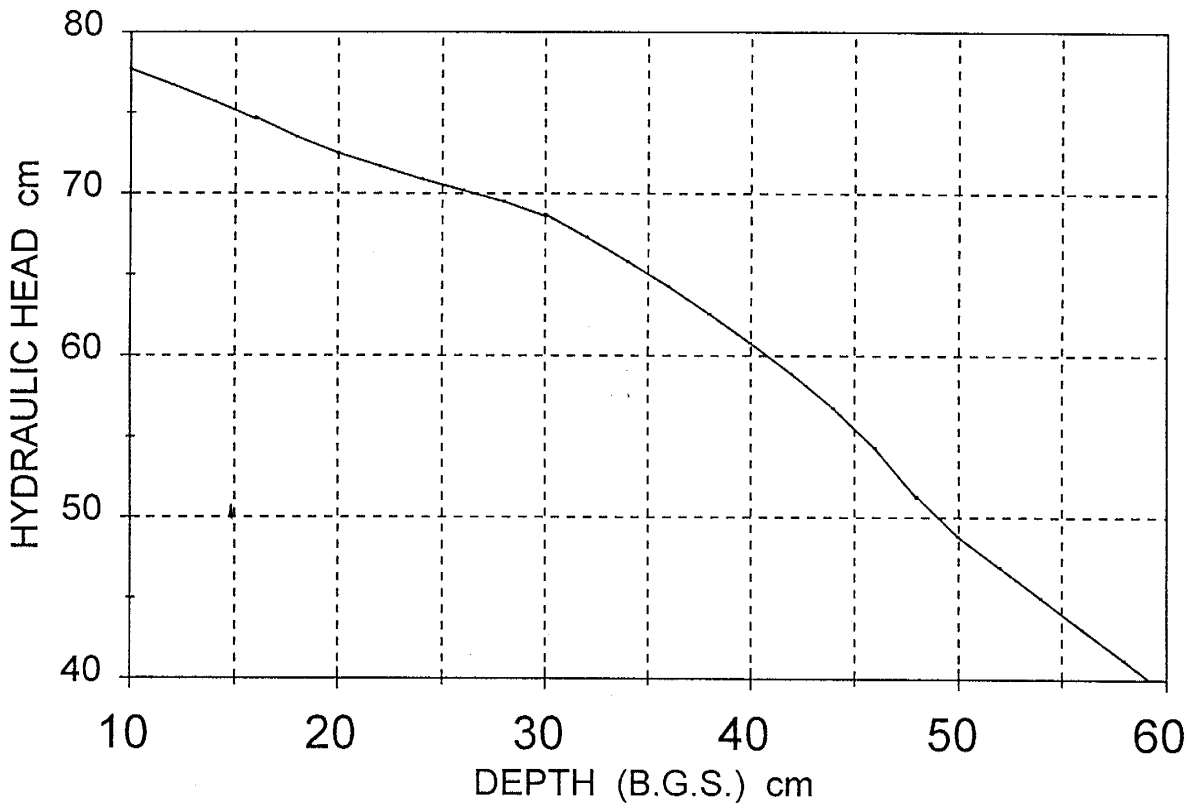


Figure 20. Computer simulation of the head with depth profile for a heterogeneous soil column using Chain_2D.

Figure 21 presents the data for an impeding stone layer of 10 centimeters. The initial trends observed in the 6.5 centimeter stone layer are again present, but they are more distinct as the thickness of the layer increases. Again layers 2 and 3 do not approximate the fitted curve or the surrounding matrix. The stony layer has higher conductivity values at similar water contents while layer 2 behaves similar to the "pseudo" low conductivity layer described above. The observed behavior is the result of processes occurring that are similar to those described for the 6.5 centimeter profile, hysteresis effects combined with errors in estimating the hydraulic gradient.

Figure 22 presents the data for an impeding layer of 16 centimeters. In this profile the thickness of the stone layer was increased to a maximum of 16 centimeters, being positioned in layers two and three. The clarity of the previously observed trends has been degraded somewhat by this change. Layers 2&3 exhibit higher conductivities at similar water contents than the fitted curve, while layer 1, now the layer immediately above the stone layer, exhibits lower conductivities at higher volumetric water contents. The observed effects are consistent throughout all of the heterogeneous profile experiments, with the degree of the hysteresis effect being related to the thickness of the stone layer and the proportion of differential wetting.

Estimation of Conductivity in Stony Soils

A comparison of several methods used to estimate the hydraulic conductivity of stony soils was performed in order to test their validity under unsaturated conditions. The equations used to accomplish this are those of Peck and Watson (1979), Bower and Rice (1984), and a simple correction, presented here as a possible new approach, based on the volume of stones present in the soil profile as a single layer. The specific equations are as follows:

UNSATURATED CONDUCTIVITY 10 CENTIMETER STONE LAYER

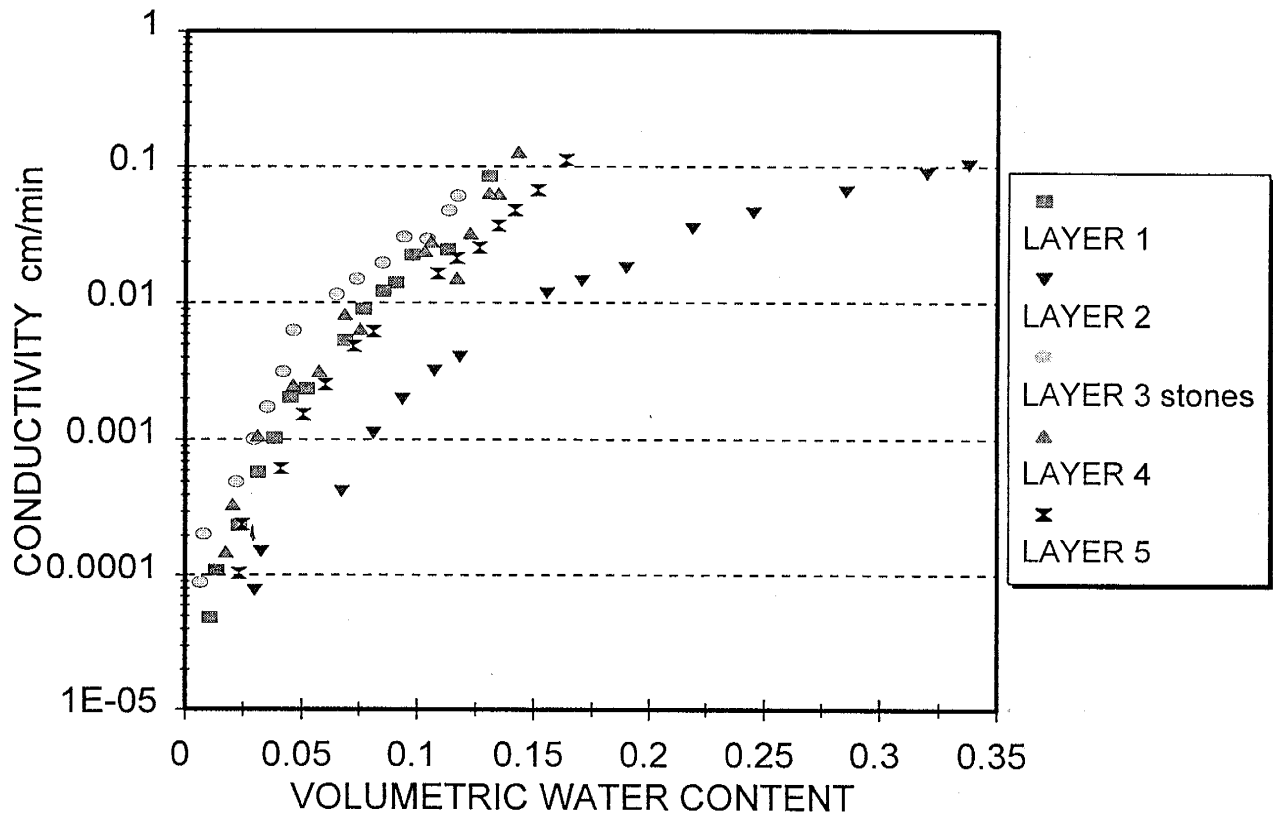


Figure 21. Unsaturated hydraulic conductivity versus volumetric water content, (θ_v), heterogeneous soil profile. Water content corrected for the volume of stones present.

UNSATURATED CONDUCTIVITY 16 CENTIMETER STONE LAYER

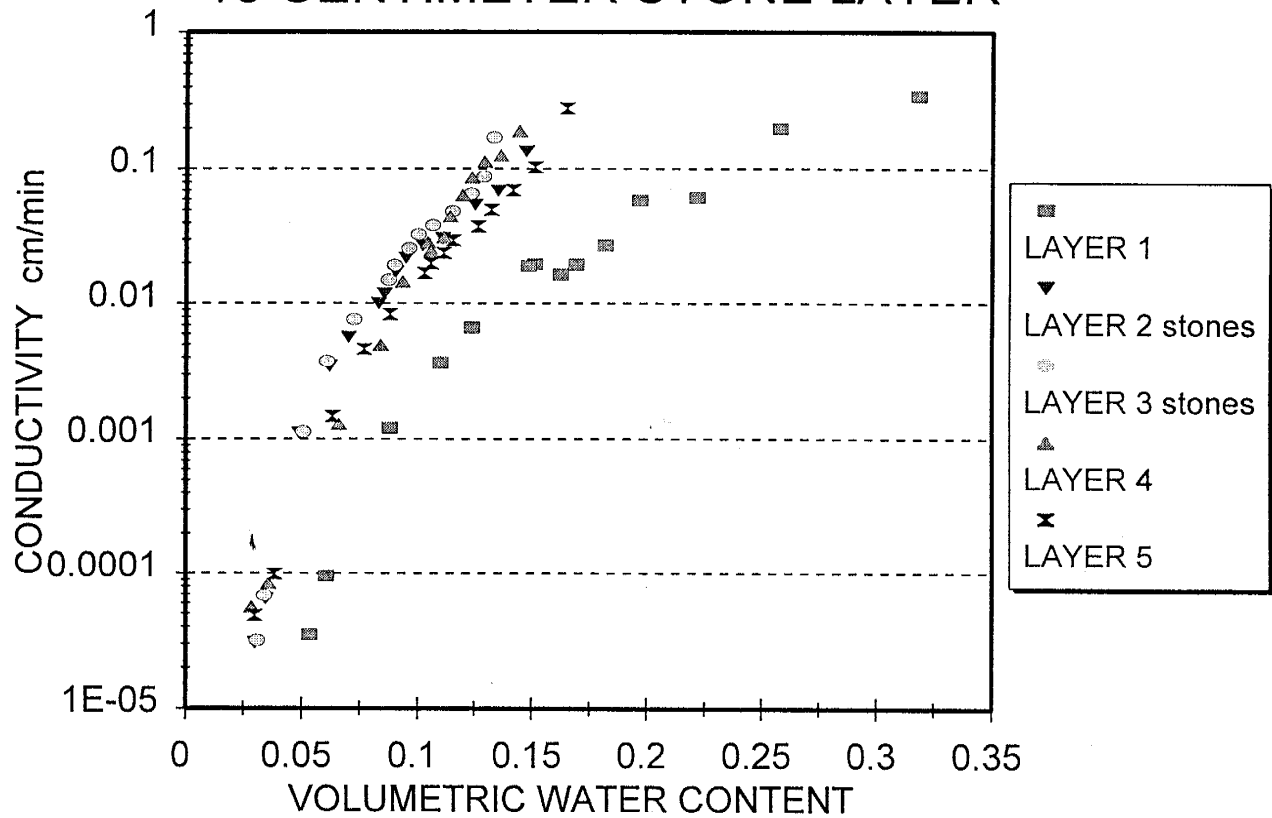


Figure 22. Unsaturated hydraulic conductivity versus volumetric water content, (θ_v), heterogeneous soil profile. Water content corrected for the volume of stones present.

$$K_b = K_s \frac{2(1-V_{st})}{2+V_{st}} \quad (3)$$

after Peck and Watson (1979) where:

K_b = bulk hydraulic conductivity of stony soil,

K_s = hydraulic conductivity of the soil alone, and

V_{st} = volume fraction of stones.

$$K_b = K_s \frac{e_b}{e_s} \quad (4)$$

after Bower and Rice (1984) where:

K_b = bulk hydraulic conductivity of the sand and gravel mixture,

K_s = hydraulic conductivity of the sand fraction alone,

e_b = bulk void ratio (volume of the voids divided by the volume of the solids) of the sand and stone mixture, and

e_s = void ratio of the sand.

$$K_b = K_s \times (1 - V_{st}) \quad (5)$$

and a simple volume correction where:

K_b = bulk unsaturated hydraulic conductivity of the stony soil

K_s = unsaturated hydraulic conductivity of the matrix, and

V_{st} = volume of stones in the soil as a single layer.

The heterogeneous profile containing the ten centimeter stone layer was selected for comparing the various methods, a result of it being the heterogeneous soil profile of intermediate thickness. Tables 2 & 3 present values of unsaturated hydraulic conductivity for the homogeneous soil profile at a series of tensions and volumetric water contents, and the resulting values calculated for the heterogeneous profiles using the above mentioned equations. These conductivity estimates are presented in Figures 23 & 24 as plots of the $K(\Psi)$ and $K(\theta)$ functions.

Figure 23 shows that the relationship of unsaturated hydraulic conductivity to tension in the stony soil is only roughly approximated by each of the methods. Each of the methods fail to accurately predict the observed data at high soil water tensions. All of the methods underestimate the unsaturated hydraulic conductivity at high tensions, although all of the estimates are within an order of magnitude of the observed data. The simple volumetric correction being applied to the conductivity of the homogeneous soil is unable to accurately predict the effects of hysteresis in the soil profile. This is best illustrated by the consistent increase in the ratio of the heterogeneous conductivity to that of homogeneous conductivity, Table 2. This ratio increases with increasing soil water tension, while the volumetric correction is constant throughout all changes in tension.

Figure 24 shows the relationship of unsaturated conductivity to water content for the same soil profile. Similar to the $K(\Psi)$ plot each of the methods behave distinctly different from one another. However, unlike the $K(\Psi)$ plot, estimates of conductivity as a function of water content underestimate the unsaturated conductivity at high water contents, i.e. low tension. The method of Peck & Watson and Bower & Rice behave in a similar fashion, under estimating the hydraulic conductivity for the stony profile by nearly identical amounts. It is very interesting to note that by simply multiplying the homogeneous conductivity values by the simple correction for the

Tension cm H ₂ O	K _{hmo} cm/min	K _{st} cm/min	K _{st} /K _{hmo} cm/min	K _{hmo} * (1-V _{ST}) cm/min	Peck&Watson cm/min	Bower&Rice cm/min
20	0.50	0.18	0.36	0.225	0.167	0.152
30	0.17	0.07	0.41	0.0765	0.0569	0.0517
40	0.03	0.017	0.56	0.0135	0.01004	0.0091
50	0.003	0.003	1.0	0.00135	0.001	9.1E -04
60	4.5E -04	7.0E -04	1.55	2.03E -04	1.5E -04	1.4E -04
70	7.5E -05	1.0E -04	1.33	3.37E -05	2.5E -05	2.3E -05

K_{hmo} : Unsaturated hydraulic conductivity homogeneous profile.

K_{st} : Unsaturated hydraulic conductivity heterogeneous profile.

Table 2. Calculation of unsaturated hydraulic conductivity for various tensions using the equations of Peck and Watson (1975), Bower and Rice (1984) and a simple correction for the volume of stones present in the soil profile.

Theta %	K_{hmo} cm/min	K_{st} cm/min	K_{st}/K_{hmo} cm/min	$K_{hmo} * (1-V_{ST})$ cm/min	Peck&Watson cm/min	Bower&Rice cm/min
2.5	0.0004	0.00018	0.45	0.00018	0.00013	0.00012
5	0.0035	0.0017	0.48	0.00158	0.00117	0.00106
7.5	0.01	0.0065	0.65	0.0045	0.00335	0.00304
10	0.03	0.015	0.50	0.0135	0.01004	0.00912
12.5	0.06	0.031	0.52	0.0270	0.02008	0.01824
15	0.09	0.055	0.61	0.0405	0.03012	0.02736
17.5	0.15	0.09	0.60	0.0675	0.05019	0.0456
20	0.18	0.14	0.77	0.081	0.06023	0.05472
22.5	0.24	0.17	0.71	0.108	0.08031	0.07295
25	0.32	0.25	0.78	0.144	0.10708	0.09727

K_{hmo} : Unsaturated hydraulic conductivity homogeneous profile.

K_{st} : Unsaturated hydraulic conductivity heterogeneous profile.

Table 3. Calculation of unsaturated hydraulic conductivity for various volumetric water contents using the equations of Peck and Watson (1975), Bower and Rice (1984) and a simple correction for the volume of stones present in the soil profile.

CORRECTION OF CONDUCTIVITY FOR THE VOLUME OF STONES PRESENT

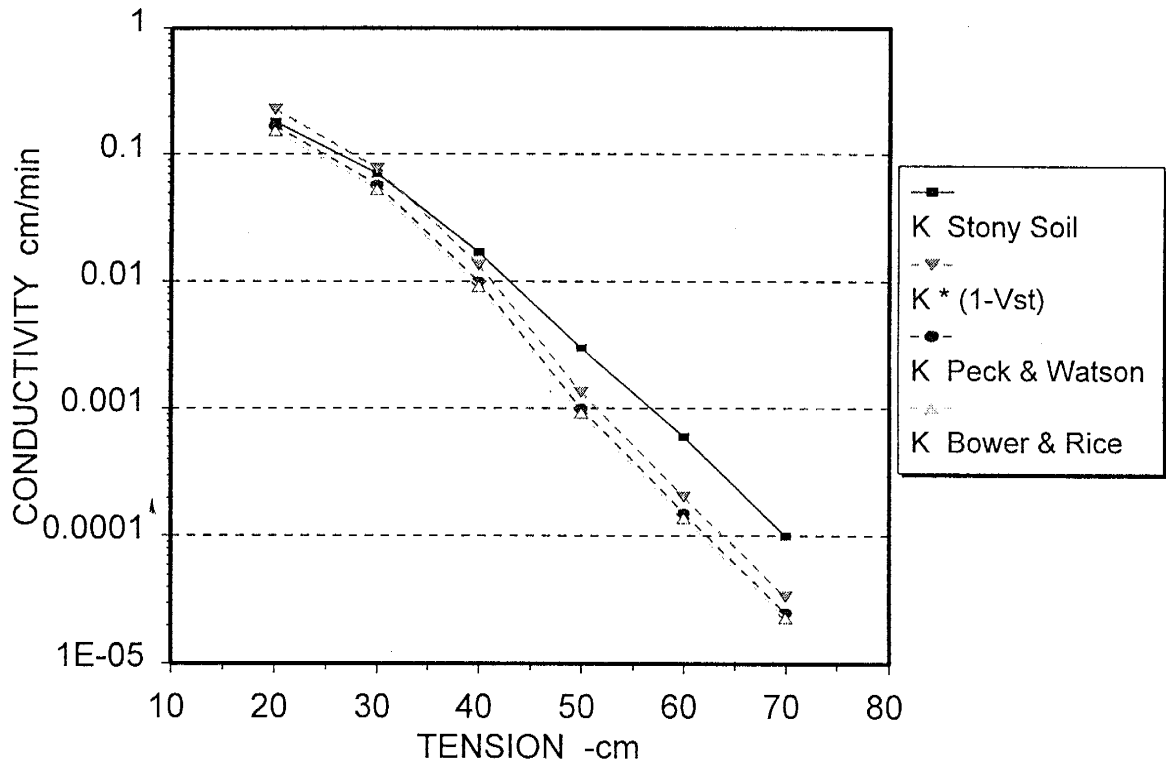


Figure 23. Correction of conductivity for the volume of stones present in the soil profile, conductivity verses tension for the 10 centimeter stone layer. A comparison of various methods.

CORRECTION OF CONDUCTIVITY FOR THE VOLUME OF STONES PRESENT

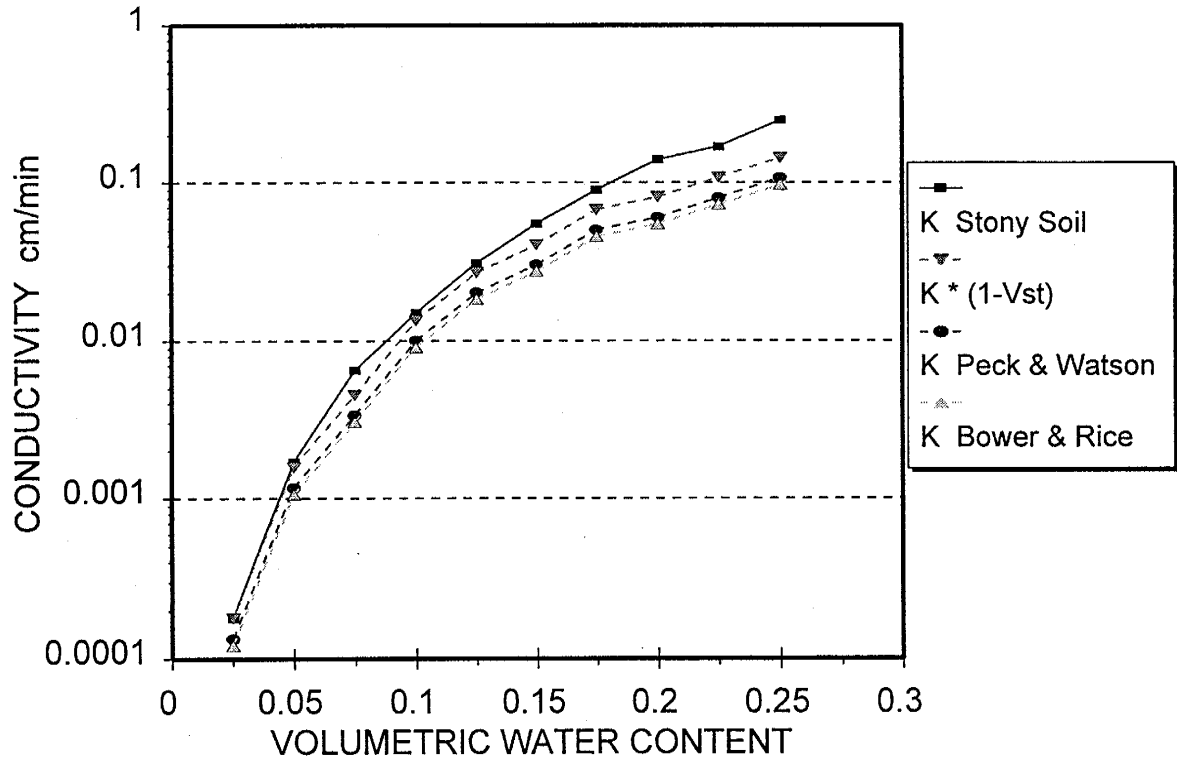


Figure 24. Correction of conductivity for the volume of stones present in the soil profile, conductivity versus volumetric water content for the 10 centimeter stone layer. A comparison of various methods.

volume of stones present in the soil profile a better prediction of the observed data is obtained. It is clear that when dealing with layered stony soils this simple volumetric correction provides an excellent estimate of the bulk unsaturated hydraulic conductivity of the soil profile.

CONCLUSION

The use of the TDR technique, tensiometry, and the instantaneous profile method proved to be valid for studying the effects of stone volume on unsaturated hydraulic conductivity in a layered stony soil. Significant findings resulting from an evaluation of the experimental approach and the heterogeneous soil profiles are as follows. First, although some studies suggest that the TDR technique may be used in stony soil with little or no loss in accuracy, [Drungil et al. (1989), Richardson et. al (1992)], the presence of stones in a soil profile may result in an improper estimation of soil water content. The presence of stones with less wet interior portions occurring in the TDR sample volume may impart significant error to the collected readings by creating a heterogeneous water distribution within the sample volume. Second, the presence of stones in a soil profile necessitate that their volume as a percentage of the soil profile be determined as well as there spacial distribution. Soils with a high volume of stones would best be studied using the TDR technique if the probes were smaller "mini-probes" similar to the ones described by Sobczuk et. al. (1992), or smaller probes constructed using the guidelines of Zegelin et al. (1992), Knight et al. (1994). Third, when utilizing the instantaneous profile method to evaluate the hydraulic character of a soil profile the analysis must be corrected for the volume of stones present in the soil profile to avoid an over estimation of the volumetric water content for the stone layer, and related underestimation of unsaturated hydraulic conductivity for this layer. Fourth, the use of the instantaneous profile method in stony layered soils requires that the lithology of the soil profile be determined during the investigation to avoid the improper identification of the low conductivity "pseudo" layer, immediately above stony layers, as a distinct lithologic horizons. The presented data

strongly suggests that unique hysteric effects occur in layered stony soils when analyzed using the instantaneous profile method which have not been addressed in previous studies. Additionally, the limitations of the instantaneous profile method, with respect to the estimation of hydraulic gradients, should be considered when using the method. Finally, a comparison of the methods used to estimate unsaturated hydraulic conductivity in stony soils indicates that a simple correction for the volume of stones present in the profile is a direct and effective method to estimate bulk unsaturated hydraulic conductivity.

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APPENDIX A

CALIBRATION OF THE TDR PROBES TO THE SEVILLETA FIELD SAND

EXPERIMENTAL PROCEDURE

APPENDIX A

EXPERIMENTAL PROCEDURE

In order to collect accurate measurements of volumetric water content it was necessary to calibrate the TDR to the field soil used for this study. This required that a series of TDR measurements be made on the soil at a number of varying, but known, water contents. The procedure used was adapted from one developed Wierenga (1993) The actual calibration was performed using the following procedure and the Sevilleta field sand.

Dry field soil was placed in a small acrylic column with a free draining base. The weights of the sand, column, base, and TDR probe were measured to determine the mass of the experimental apparatus and the dry bulk density of the sand. The column was then saturated from the bottom up by placing it in a tray of water. A TDR probe was then inserted into the column and the all components reweighed. The column was then allowed to drain over a period of days while TDR readings and sample weights were recorded. The experiment was concluded when the column has again attained its original presaturation weight. This data was then used to calculate the volumetric water content for the soil when TDR measurements were recorded.

The results were then plotted as measured TDR reading verses volumetric water content, and a linear regression fitted to the data. The equation of the line fitted to this curve provided the calibration curve for the TDR in the Sevilleta field soil, where the x-coordinate is the measured TDR reading and the returned y-intercept is the calculated volumetric water content. This equation was then used to calculate volumetric water content in all of the column experiments. Figure A-1 shows the data from one of three calibration experiments, it also shows the linear regression curve fitted to the experimental data.

A comparison of this calibration curve with the one developed by Topp et al. (1980) shows a high degree of correlation between the two when compared as dielectric constant versus volumetric water content. This comparison, presented as Figure A-2, validates the method of calibration used, and suggests that this approach is a relatively easy way to calibrate the TDR to a particular field soil one is studying.

CALIBRATION CURVE FOR THE SEVILLETA FIELD SAND

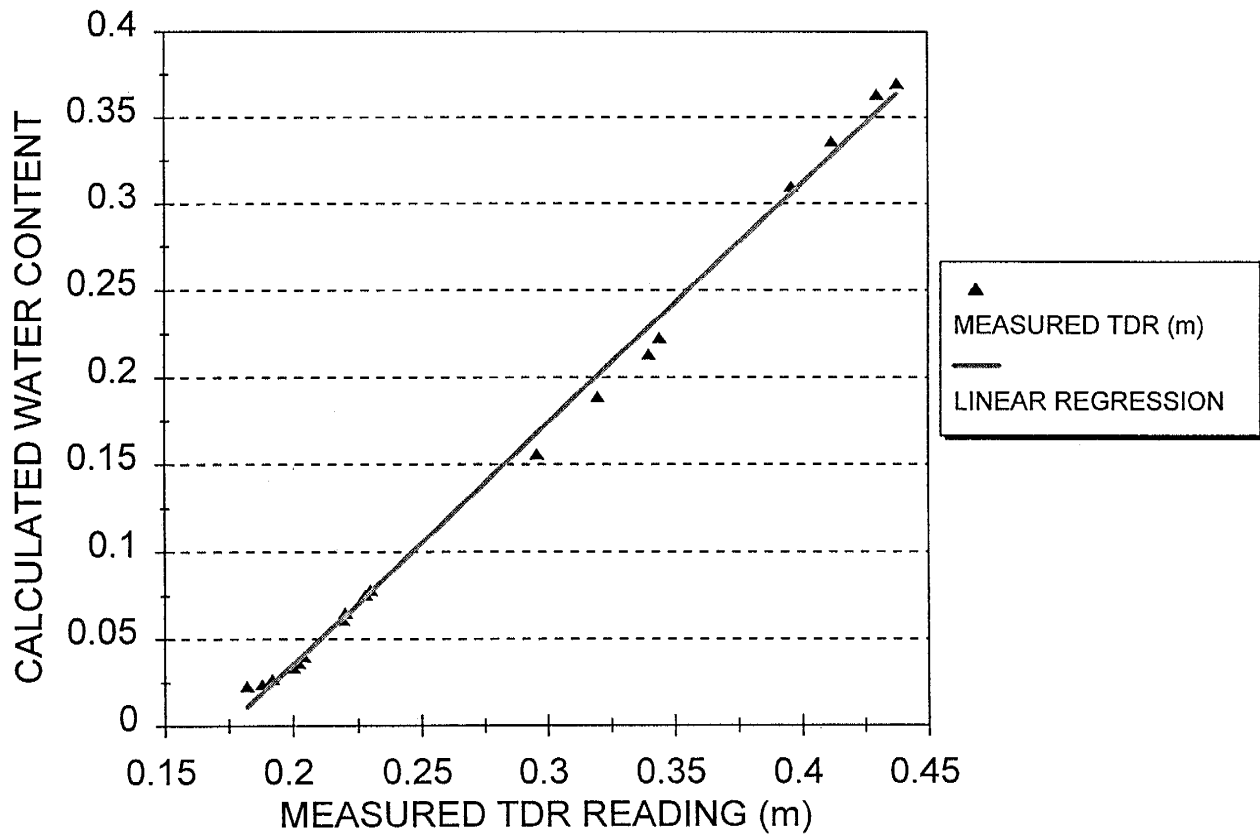


Figure A-1. TDR calibration curve for the Sevilleta field sand.

THETA CALCULATED USING TOPP'S METHOD COMPARED TO LABORATORY CALIBRATION

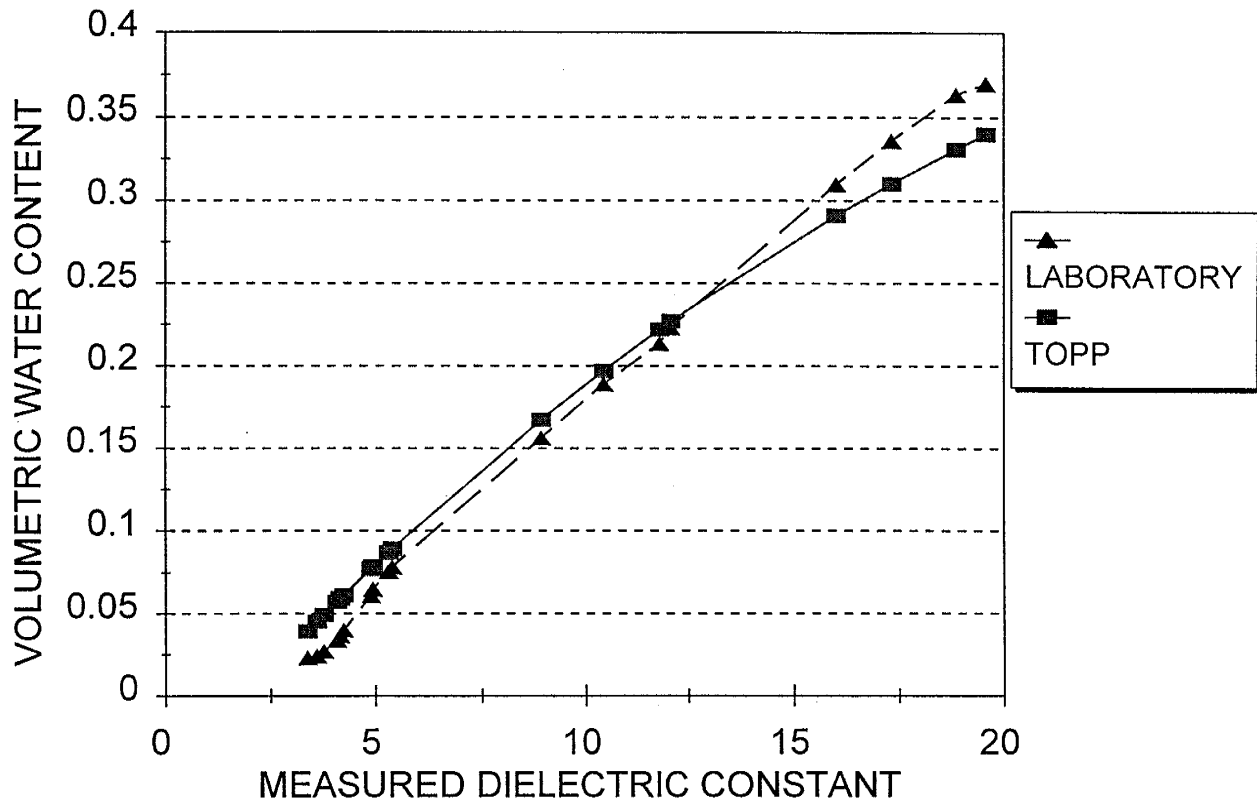


Figure A-2. Comparison of estimated volumetric water content using the equation of Topp et al. (1980) as compared to that calculated using the laboratory calibration curve determined in this study.

APPENDIX B

REVIEW OF TDR THEORY AND PRINCIPLES

APPENDIX B

TDR THEORY REVIEW

A detailed explanation of the theory and principles involved in time domain reflectometry has been presented by Topp et al. (1980), Topp & Davis (1985), while the historical development of the TDR in soil physics and its practical use is discussed by Cassel et al. (1994). Stated in the most succinct manner the TDR technique measures the velocity of propagation of a high frequency electrical signal in a dielectric medium, i.e. a moist soil, and records the amount by which this electrical pulse is attenuated in the medium. The TDR unit or cable tester, in this case the Tektronix model 1502B, initiates a step voltage pulse via its signal generator. This voltage pulse travels along the transmission line, i.e. coaxial cable, which terminates in an ordered arrangement of wave guides. The transmission line used in constructing the probes is of the same impedance as the output signal of the TDR, 50Ω , so the signal pulse travels unhindered through it. Wherever the impedance of the transmission line changes a mismatch occurs causing a portion of the signal energy to be reflected back to the cable tester unit. This pulse of reflected signal energy manifests itself as a discontinuity on the oscilloscope display. The observed discontinuity is related to the magnitude of the impedance mismatch and the dielectric property of the medium in which it occurs. The characteristic wave form seen on the display screen is due to the signal pulse encountering the impedance mismatch as it enters the wave guides of the probe, initial reflection point, and the total reflection of the signal pulse at the end of the wave guides where all of the energy is subsequently returned, final reflection point. Through a process of internal conversion the reflected voltage pulse is displayed on the oscilloscope as length on the abscissa and returned voltage on the ordinate. The apparent length of the wave guides in the soil is obtained by measuring the distance between the initial and final reflection points

of the displayed wave form. This apparent length is a function of the dielectric properties of the medium. The high dielectric constant of water , approximately 80, as compared to that of soil, approximately 3 to 7, Cassel et al. (1994), illustrates why the TDR technique is effective in determining soil water content. A medium having a relatively high dielectric constant , for example a moist soil, will return an apparent length for the probe significantly greater than its true length. This apparent length is then used to calculate the dielectric constant for the soil which can then be calculated as a function of volumetric water content. A detailed description relating the measured apparent probe length, calculated dielectric constant of the medium, and calculated volumetric water content can be found elsewhere in the literature Topp (1980), Topp & Davis (1985), Spaans & Baker (1993).

The accuracy of TDR measurements in the heterogeneous profiles depend to a great extent on what volume of soil is sampled by the TDR in the stone layer. As stated above, is this measurement representative of the real matrix water content or is it a larger volume averaged measurement including portions of the stones which are dry? The volume of soil sampled is dependent upon the electric field distribution generated around the probe and what spatial weighting function is associated with the specific probe type. Several articles have appeared in the literature that address this issue from two very different approaches. One is a theoretical approach that deals primarily with transmission line theory, while the other is an empirical approach. The work of Zegelin et al. (1989), Zegelin et al. (1992), Knight (1991), Knight (1992) and Knight et al. (1994) present detailed accounts of the principles involved in estimating the sample volume and spatial weighting functions for probes of various configurations. This information would permit one to develop the necessary equations to estimate the electric field distributions and spatial weighting functions for two or three

wire probes given idealized conditions of probe construction and instrument performance. However, a specific sample volume or range of influence for three wire probes similar to the ones used is not provided. This approach did not provide a readily discernable solution to the problem relating to the experimental procedure used. Empirical approaches were examined to determine if an acceptable experimental procedure could be adapted to the laboratory setup. Empirical approaches used by Topp & Davis (1985), Baker & Lascano (1989), and Richardson et al. (1992) provided a frame work for the experimental procedures detailed in the text of the paper.

APPENDIX C

EXPERIMENTAL DATA: HOMOGENEOUS SOIL PROFILE

SEVILLETA SAND - NO STONES

COLUMN EXPERIMENT #6 HOMOGENEOUS SEVILLETIA SAND							
TENSIO METER READINGS mbar							
MINUTES	T1	T2	T3	T4	T5	T6	
1	-20.5	-18	-16	-15	-13	-11	
15	-38.5	-36	-35	-32.5	-31	-26.5	
30	-48.5	-46.5	-44.5	-43	-43.5	-38.5	
45	-52.5	-49.5	-48.5	-48	-48	-46	
60	-54	-52.5	-51	-50	-50	-48.5	
90	-58	-56.5	-54	-52.5	-53.5	-51.5	
120	-60.5	-59	-56.5	-55	-54.5	-52.5	
150	-61.5	-60	-57.5	-56.5	-56	-54.5	
180	-63.5	-61.5	-59	-58	-58	-55.5	
210	-64.5	-62.5	-59.5	-59	-58	-57	
270	-66.5	-64.5	-60.5	-60.5	-59.5	-58	
390	-69	-67.5	-64.5	-63.5	-63	-60	
510	-71.5	-70	-67	-65.5	-65	-61	
630	-73.5	-71.5	-67.5	-68	-65.5	-62.5	
1200	-76	-76	-73	-72.5	-70	-64	
1500	-77	-77	-74.5	-74	-71	-64	
2880	-83	-82	-78.5	-77	-73.5	-65	
4380	-85.5	-85	-81.5	-79	-72.5	-64	
5610	-87	-86.5	-83.5	-81	-74.5	-65	
TENSIO METER READINGS cm H2O							
MINUTES	T1	T2	T3	T4	T5	T6	
1	-20.9039	-18.3546	-16.3152	-15.2955	-13.2561	-11.2167	
15	-39.2585	-36.7092	-35.6895	-33.1403	-31.6107	-27.0221	
30	-49.4555	-47.4161	-45.3767	-43.8471	-44.357	-39.2585	
45	-53.5343	-50.4752	-49.4555	-48.9456	-48.9456	-46.9062	
60	-55.0638	-53.5343	-52.0047	-50.985	-50.985	-49.4555	
90	-59.1426	-57.6131	-55.0638	-53.5343	-54.554	-52.5146	
120	-61.6919	-60.1623	-57.6131	-56.0835	-55.5737	-53.5343	
150	-62.7116	-61.182	-58.6328	-57.6131	-57.1032	-55.5737	
180	-64.751	-62.7116	-60.1623	-59.1426	-59.1426	-56.5934	
210	-65.7707	-63.7313	-60.6722	-60.1623	-59.1426	-58.1229	
270	-67.8101	-65.7707	-61.6919	-61.6919	-60.6722	-59.1426	
390	-70.3593	-68.8298	-65.7707	-64.751	-64.2411	-61.182	
510	-72.9086	-71.379	-68.3199	-66.7904	-66.2805	-62.2017	
630	-74.948	-72.9086	-68.8298	-69.3396	-66.7904	-63.7313	
1200	-77.4972	-77.4972	-74.4381	-73.9283	-71.379	-65.2608	
1500	-78.5169	-78.5169	-75.9677	-75.4578	-72.3987	-65.2608	
2880	-84.6351	-83.6154	-80.0465	-78.5169	-74.948	-66.2805	
4380	-87.1844	-86.6745	-83.1056	-80.5563	-73.9283	-65.2608	
5610	-88.7139	-88.2041	-85.145	-82.5957	-75.9677	-66.2805	

CORRECTED TENSIO METER READINGS AT CUPS							
MINUTES	T1	T2	T3	T4	T5	T6	
1	-7.40385	-4.8546	-2.8152	-1.7955	0.2439	2.2833	
15	-25.7585	-23.2092	-22.1895	-19.6403	-18.1107	-13.5221	
30	-35.9555	-33.9161	-31.8767	-30.3471	-30.857	-25.7585	
45	-40.0343	-36.9752	-35.9555	-35.4456	-35.4456	-33.4062	
60	-41.5638	-40.0343	-38.5047	-37.485	-37.485	-35.9555	
90	-45.6426	-44.1131	-41.5638	-40.0343	-41.054	-39.0146	
120	-48.1919	-46.6623	-44.1131	-42.5835	-42.0737	-40.0343	
150	-49.2116	-47.682	-45.1328	-44.1131	-43.6032	-42.0737	
180	-51.251	-49.2116	-46.6623	-45.6426	-45.6426	-43.0934	
210	-52.2707	-50.2313	-47.1722	-46.6623	-45.6426	-44.6229	
270	-54.3101	-52.2707	-48.1919	-48.1919	-47.1722	-45.6426	
390	-56.8593	-55.3298	-52.2707	-51.251	-50.7411	-47.682	
510	-59.4086	-57.879	-54.8199	-53.2904	-52.7805	-48.7017	
630	-61.448	-59.4086	-55.3298	-55.8396	-53.2904	-50.2313	
1200	-63.9972	-63.9972	-60.9381	-60.4283	-57.879	-51.7608	
1500	-65.0169	-65.0169	-62.4677	-61.9578	-58.8987	-51.7608	
2880	-71.1351	-70.1154	-66.5465	-65.0169	-61.448	-52.7805	
4380	-73.6844	-73.1745	-69.6056	-67.0563	-60.4283	-51.7608	
5610	-75.2139	-74.7041	-71.645	-69.0957	-62.4677	-52.7805	
HYDRAULIC HEAD AT THE TENSIO METER CUPS H							
MINUTES	T1	T2	T3	T4	T5	T6	
1	-18.6539	-26.1046	-34.8152	-43.0455	-51.7561	-59.7167	
15	-37.0085	-44.4592	-54.1895	-60.8903	-70.1107	-75.5221	
30	-47.2055	-55.1661	-63.8767	-71.5971	-82.857	-87.7585	
45	-51.2843	-58.2252	-67.9555	-76.6956	-87.4456	-95.4062	
60	-52.8138	-61.2843	-70.5047	-78.735	-89.485	-97.9555	
90	-56.8926	-65.3631	-73.5638	-81.2843	-93.054	-101.015	
120	-59.4419	-67.9123	-76.1131	-83.8335	-94.0737	-102.034	
150	-60.4616	-68.932	-77.1328	-85.3631	-95.6032	-104.074	
180	-62.501	-70.4616	-78.6623	-86.8926	-97.6426	-105.093	
210	-63.5207	-71.4813	-79.1722	-87.9123	-97.6426	-106.623	
270	-65.5601	-73.5207	-80.1919	-89.4419	-99.1722	-107.643	
390	-68.1093	-76.5798	-84.2707	-92.501	-102.741	-109.682	
510	-70.6586	-79.129	-86.8199	-94.5404	-104.781	-110.702	
630	-72.698	-80.6586	-87.3298	-97.0896	-105.29	-112.231	
1200	-75.2472	-85.2472	-92.9381	-101.678	-109.879	-113.761	
1500	-76.2669	-86.2669	-94.4677	-103.208	-110.899	-113.761	
2880	-82.3851	-91.3654	-98.5465	-106.267	-113.448	-114.781	
4380	-84.9344	-94.4245	-101.606	-108.306	-112.428	-113.761	
5610	-86.4639	-95.9541	-103.645	-110.346	-114.468	-114.781	

WATER CONTENTS via TDR						
MINUTES	TDR #1	THETA 1	TDR #2	THETA 2	TDR#3	THETA 3
1	0.364	0.261682	0.384	0.289267	0.364	0.261682
15	0.324	0.206514	0.35	0.242374	0.34	0.228581
30	0.262	0.121003	0.272	0.134795	0.29	0.159621
45	0.244	0.096177	0.252	0.107211	0.27	0.132037
60	0.234	0.082385	0.244	0.096177	0.254	0.109969
90	0.224	0.068593	0.234	0.082385	0.244	0.096177
120	0.218	0.060318	0.228	0.07411	0.24	0.09066
150	0.212	0.052043	0.216	0.057559	0.23	0.076868
180	0.21	0.049284	0.216	0.057559	0.226	0.071351
210	0.206	0.043767	0.212	0.052043	0.222	0.065835
270	0.202	0.03825	0.206	0.043767	0.22	0.063076
390	0.198	0.032734	0.202	0.03825	0.21	0.049284
510	0.196	0.029975	0.198	0.032734	0.206	0.043767
630	0.194	0.027217	0.196	0.029975	0.202	0.03825
1200	0.19	0.0217	0.192	0.024458	0.198	0.032734
1500	0.182	0.010666	0.188	0.018941	0.196	0.029975
2880	0.18	0.007908	0.184	0.013425	0.19	0.0217
4380	0.178	0.005149	0.18	0.007908	0.186	0.016183
5610	0.178	0.005149	0.18	0.007908	0.186	0.016183
MINUTES	TDR #4	THETA 4	TDR #5	THETA 5	TDR #6	THETA 6
1	0.414	0.330643	0.422	0.341677	0.426	0.347193
15	0.384	0.289267	0.398	0.308576	0.404	0.316851
30	0.318	0.198239	0.332	0.217548	0.36	0.256166
45	0.26	0.118245	0.272	0.134795	0.314	0.192722
60	0.242	0.093419	0.256	0.112728	0.298	0.170655
90	0.234	0.082385	0.246	0.098936	0.28	0.145829
120	0.232	0.079627	0.24	0.09066	0.272	0.134795
150	0.22	0.063076	0.234	0.082385	0.264	0.123761
180	0.22	0.063076	0.224	0.068593	0.258	0.115486
210	0.216	0.057559	0.222	0.065835	0.252	0.107211
270	0.214	0.054801	0.222	0.065835	0.25	0.104453
390	0.208	0.046526	0.216	0.057559	0.242	0.093419
510	0.204	0.041009	0.214	0.054801	0.242	0.093419
630	0.204	0.041009	0.21	0.049284	0.24	0.09066
1200	0.198	0.032734	0.204	0.041009	0.24	0.09066
1500	0.194	0.027217	0.2	0.035492	0.236	0.085144
2880	0.19	0.0217	0.196	0.029975	0.232	0.079627
4380	0.19	0.0217	0.198	0.032734	0.23	0.076868
5610	0.19	0.0217	0.198	0.032734	0.23	0.076868

SLOPE OF WETTNESS CURVES dTH/dt						
MINUTES	dTH/dt 1	dTH/dt 2	dTH/dt 3	dTH/dt 4	dTH/dt 5	dTH/dt 6
1	0.017587	-0.21604	-0.1189	-0.25823	-0.22126	-2293.9
15	0.003901	0.011903	0.007469	0.015332	0.013611	0.010979
30	0.001852	0.003259	0.002714	0.004087	0.004132	0.003706
45	0.0013	0.001534	0.001431	0.001899	0.002014	0.001905
60	0.000778	0.00091	0.000904	0.001117	0.001215	0.001177
90	0.000448	0.000444	0.000473	0.00054	0.000604	0.000591
120	0.000298	0.00027	0.0003	0.000327	0.000371	0.000361
150	0.000215	0.000185	0.000211	0.000224	0.000252	0.000245
180	0.000165	0.000137	0.000159	0.000165	0.00019	0.000179
210	0.000131	0.000106	0.000125	0.000127	0.000148	0.000137
270	8.93E-05	7.07E-05	8.48E-05	8.45E-05	9.88E-05	8.82E-05
390	5.07E-05	3.94E-05	4.48E-05	4.69E-05	5.53E-05	4.64E-05
510	3.34E-05	2.59E-05	3.24E-05	3.08E-05	3.65E-05	2.90E-05
630	2.40E-05	1.87E-05	2.37E-05	2.22E-05	2.65E-05	2.01E-05
1200	8.64E-06	7.12E-06	9.25E-06	8.40E-06	1.01E-05	6.57E-06
1500	6.05E-06	5.13E-06	6.71E-06	6.04E-06	7.28E-06	4.47E-06
2880	2.13E-06	2.00E-06	2.67E-06	2.35E-06	1.58E-06	1.48E-06
4380	1.08E-06	1.11E-06	1.49E-06	1.30E-06	1.12E-06	7.38E-07
5610	7.28E-07	7.84E-07	1.06E-06	9.18E-07	2.85E-06	4.93E-07
WATER CONTENT dTH/dt TIMES THE THICKNESS OF THE LAYER						
MINUTES	H2O 1	H2O 2	H2O 3	H2O 4	H2O 5	H2O 6
1	0.351742	-2.16039	-1.18903	-2.58235	-2.21259	-22939
15	0.078028	0.119025	0.074693	0.153319	0.136109	0.109792
30	0.037038	0.03259	0.027144	0.04087	0.041318	0.03706
45	0.026	0.015344	0.014313	0.018991	0.020139	0.019048
60	0.015566	0.009098	0.009037	0.011172	0.012153	0.011769
90	0.00896	0.004438	0.004728	0.0054	0.006037	0.005911
120	0.005958	0.002704	0.002995	0.003272	0.003713	0.003606
150	0.004308	0.001854	0.002108	0.002235	0.002521	0.002453
180	0.00329	0.001369	0.001586	0.001646	0.001899	0.001788
210	0.002612	0.001062	0.001249	0.001274	0.001479	0.001367
270	0.001786	0.000707	0.000848	0.000845	0.000988	0.000882
390	0.001015	0.000394	0.000448	0.000469	0.000553	0.000464
510	0.000668	0.000259	0.000324	0.000308	0.000365	0.00029
630	0.00048	0.000187	0.000237	0.000222	0.000265	0.000201
1200	0.000173	7.12E-05	9.25E-05	8.4E-05	0.000101	6.57E-05
1500	0.000121	5.13E-05	6.71E-05	6.04E-05	7.28E-05	4.47E-05
2880	4.25E-05	2E-05	2.67E-05	2.35E-05	1.58E-05	1.48E-05
4380	2.17E-05	1.11E-05	1.49E-05	1.3E-05	1.12E-05	7.38E-06
5610	1.46E-05	7.84E-06	1.06E-05	9.18E-06	2.85E-05	4.93E-06

SLOPE OF HEAD WITH DEPTH CURVE dH/dz						
MINUTES						
1	0.824041	0.824041	0.824041	0.824041	0.824041	0.824041
15	0.78321	0.78321	0.78321	0.78321	0.78321	0.78321
30	0.83237	0.83237	0.83237	0.83237	0.83237	0.83237
45	0.899241	0.899241	0.899241	0.899241	0.899241	0.899241
60	0.903621	0.903621	0.903621	0.903621	0.903621	0.903621
90	0.883421	0.883421	0.883421	0.883421	0.883421	0.883421
120	0.848669	0.848669	0.848669	0.848669	0.848669	0.848669
150	0.868965	0.868965	0.868965	0.868965	0.868965	0.868965
180	0.858745	0.858745	0.858745	0.858745	0.858745	0.858745
210	0.858937	0.858937	0.858937	0.858937	0.858937	0.858937
270	0.841561	0.841561	0.841561	0.841561	0.841561	0.841561
390	0.83553	0.83553	0.83553	0.83553	0.83553	0.83553
510	0.807934	0.807934	0.807934	0.807934	0.807934	0.807934
630	0.797906	0.797906	0.797906	0.797906	0.797906	0.797906
1200	1.166719	0.926453	0.797029	0.712655	0.651894	0.603309
1500	1.275822	0.949546	0.783085	0.67852	0.605326	0.548153
2880	1.231703	0.871951	0.685473	0.569634	0.489955	0.428911
4380	1.457801	0.848679	0.585443	0.440171	0.34885	0.283948
5610	1.483246	0.874968	0.587874	0.426936	0.326467	0.256221
FLUX TROUGH EACH LAYER						
MINUTES	Q1	Q2	Q3	Q4	Q5	Q6
1	0.351742	-1.80865	-2.99768	-5.58003	-7.79262	-22946.8
15	0.078028	0.197053	0.271747	0.425066	0.561174	0.670966
30	0.037038	0.069628	0.096773	0.137642	0.178961	0.216021
45	0.026	0.041344	0.055657	0.074648	0.094787	0.113835
60	0.015566	0.024664	0.033701	0.044873	0.057026	0.068794
90	0.00896	0.013398	0.018127	0.023527	0.029564	0.035475
120	0.005958	0.008662	0.011657	0.014928	0.018641	0.022248
150	0.004308	0.006162	0.00827	0.010506	0.013027	0.015479
180	0.00329	0.004659	0.006245	0.007891	0.00979	0.011577
210	0.002612	0.003674	0.004923	0.006197	0.007676	0.009043
270	0.001786	0.002493	0.003341	0.004186	0.005174	0.006056
390	0.001015	0.001409	0.001857	0.002326	0.002879	0.003343
510	0.000668	0.000928	0.001252	0.00156	0.001925	0.002216
630	0.00048	0.000667	0.000904	0.001126	0.001391	0.001592
1200	0.000173	0.000244	0.000336	0.00042	0.000521	0.000587
1500	0.000121	0.000172	0.000239	0.0003	0.000373	0.000417
2880	4.25E-05	6.26E-05	8.93E-05	0.000113	0.000129	0.000143
4380	2.17E-05	3.28E-05	4.77E-05	6.07E-05	7.18E-05	7.92E-05
5610	1.46E-05	2.24E-05	3.3E-05	4.22E-05	7.07E-05	7.56E-05

SOIL WATER RETENTION DATA HOMOGENEOUS PROFILE					
FITTED CURVE			LAYER 1		
THETA	TENSION		THETA	TENSION	
0.0016	136.8		0.206514	25.75845	
0.0033	112.2		0.121003	35.95545	
0.0065	91.96		0.096177	40.03425	
0.013	75.28		0.082385	41.5638	
0.0195	66.87		0.068593	45.6426	
0.026	61.41		0.060318	48.19185	
0.0326	57.44		0.052043	49.21155	
0.0391	54.34		0.049284	51.25095	
0.0456	51.81		0.043767	52.27065	
0.0521	49.68		0.03825	54.31005	
0.0586	47.84		0.032734	56.8593	
0.0651	46.22		0.029975	59.40855	
0.0716	44.78		0.027217	61.44795	
0.0781	43.48		0.0217	63.9972	
0.0846	42.28		0.010666	65.0169	
0.0977	40.17				
0.1042	39.21				LAYER 2
0.1107	38.31		THETA	TENSION	
0.1172	37.46		0.242374	23.2092	
0.1237	36.65		0.134795	33.91605	
0.1302	35.88		0.107211	36.97515	
0.1367	35.13		0.096177	40.03425	
0.1432	34.41		0.082385	44.11305	
0.1498	33.72		0.07411	46.6623	
0.1563	33.04		0.057559	47.682	
0.1628	32.39		0.057559	49.21155	
0.1693	31.74		0.052043	50.23125	
0.1758	31.11		0.043767	52.27065	
0.1823	30.48		0.03825	55.32975	
0.1888	29.86		0.032734	57.879	
0.1953	29.25		0.029975	59.40855	
0.2018	28.63		0.024458	63.9972	
0.2084	28.02		0.018941	65.0169	
0.2149	27.4		0.013425	70.1154	
0.2214	26.78		0.007908	73.1745	
0.2279	26.14				
0.2344	25.5				
0.2409	24.83				
0.2474	24.15				
0.2539	23.43				
0.2604	22.68				
0.267	21.88				
0.2735	21.01				
0.28	20.05				
0.2865	18.97				

0.293		17.7					
0.2995		16.08					
0.306		13.71					
0.3093		11.72					
	LAYER 3			LAYER 5			
THETA		TENSION		THETA		TENSION	
0.228581		22.1895		0.308576		18.1107	
0.159621		31.87665		0.217548		30.85695	
0.132037		35.95545		0.134795		35.4456	
0.109969		38.5047		0.112728		37.485	
0.096177		41.5638		0.098936		41.05395	
0.09066		44.11305		0.09066		42.07365	
0.076868		45.13275		0.082385		43.6032	
0.071351		46.6623		0.068593		45.6426	
0.065835		47.17215		0.065835		45.6426	
0.063076		48.19185		0.065835		47.17215	
0.049284		52.27065		0.057559		50.7411	
0.043767		54.8199		0.054801		52.7805	
0.03825		55.32975		0.049284		53.29035	
0.032734		60.9381		0.041009		57.879	
0.029975		62.46765		0.035492		58.8987	
0.0217		66.54645		0.029975		61.44795	
				0.032734		60.42825	
				0.032734		62.46765	
	LAYER 4			LAYER 6			
THETA		TENSION		THETA		TENSION	
0.330643		1.7955		0.316851		13.52205	
0.289267		19.64025		0.256166		25.75845	
0.198239		30.3471		0.192722		33.4062	
0.118245		35.4456		0.170655		35.95545	
0.093419		37.485		0.145829		39.01455	
0.082385		40.03425		0.134795		40.03425	
0.079627		42.5835					
0.063076		44.11305					
0.063076		45.6426					
0.057559		46.6623					
0.054801		48.19185					
0.046526		51.25095					
0.041009		53.29035					
0.041009		55.8396					
0.032734		60.42825					
0.027217		61.9578					
0.0217		65.0169					
0.0217		67.0563					
0.0217		69.0957					

HYDRAULIC CONDUCTIVITY TENSION DATA HOMOGENEOUS PROFILE					
FITTED CURVE			LAYER 1		
TENSION		K	TENSION		K
123.6		9.47E-08	35.95545		0.044497
103.1		7.97E-07	40.03425		0.028913
86.02		6.7E-06	41.5638		0.017226
71.64		5.65E-05	45.6426		0.010142
64.29		0.000197	48.19185		0.00702
59.47		0.000477	49.21155		0.004958
55.94		0.00095	51.25095		0.003831
53.17		0.001668	52.27065		0.003041
50.9		0.002687	54.31005		0.002123
48.97		0.004064	56.8593		0.001215
47.31		0.005857	59.40855		0.000827
45.84		0.008127	61.44795		0.000601
44.52		0.01093	63.9972		0.000148
43.33		0.01435	65.0169		9.48E-05
42.24		0.01842	71.1351		3.45E-05
41.23		0.02324	73.68435		1.49E-05
40.28		0.02886	75.2139		9.82E-06
39.4		0.03536			
38.57		0.04281			
			LAYER 2		
			TENSION		K
37.78		0.0513	23.2092		0.251597
37.03		0.06089	33.91605		0.083651
36.3		0.07168	36.97515		0.045977
35.61		0.08375	40.03425		0.027294
34.94		0.09719	44.11305		0.015166
34.29		0.1121	46.6623		0.010206
33.65		0.1286	47.682		0.007091
33.03		0.1467	49.21155		0.005425
32.42		0.1667	50.23125		0.004278
31.82		0.1885	52.27065		0.002962
31.23		0.2123	55.32975		0.001686
30.64		0.2383	57.879		0.001148
30.06		0.2666	59.40855		0.000836
29.47		0.2973	63.9972		0.000263
28.89		0.3307	65.0169		0.000181
28.3		0.3668	70.1154		7.17E-05
27.7		0.4059	73.1745		3.86E-05
27.09		0.4482	74.70405		2.56E-05
26.47		0.4939			
25.83		0.5433			
25.17		0.5967			
24.47		0.6545			
23.74		0.7171			
22.96		0.7849			
22.12		0.8588			
21.18		0.9394			

20.12		1.028					
18.85		1.126					
17.25		1.237					
14.87		1.366					
12.85		1.444					
	LAYER 3				LAYER 5		
TENSION		K		TENSION		K	
22.1895		0.346965		18.1107		0.716505	
31.87665		0.116261		30.85695		0.215001	
35.95545		0.061893		35.4456		0.105408	
38.5047		0.037295		37.485		0.063108	
41.5638		0.020519		41.05395		0.033465	
44.11305		0.013735		42.07365		0.021965	
45.13275		0.009517		43.6032		0.014991	
46.6623		0.007272		45.6426		0.0114	
47.17215		0.005731		45.6426		0.008936	
48.19185		0.00397		47.17215		0.006148	
52.27065		0.002222		50.7411		0.003446	
54.8199		0.001549		52.7805		0.002383	
55.32975		0.001133		53.29035		0.001743	
60.9381		0.000422		57.879		0.0008	
62.46765		0.000306		58.8987		0.000616	
66.54645		0.00013		61.44795		0.000262	
69.60555		8.14E-05		60.42825		0.000206	
71.64495		5.62E-05		62.46765		0.000217	
	LAYER 4						
TENSION		K					
19.64025		0.542723					
30.3471		0.165362					
35.4456		0.083012					
37.485		0.049659					
40.03425		0.026632					
42.5835		0.01759					
44.11305		0.01209					
45.6426		0.009188					
46.6623		0.007215					
48.19185		0.004974					
51.25095		0.002784					
53.29035		0.001931					
55.8396		0.001411					
60.42825		0.00059					
61.9578		0.000442					
65.0169		0.000198					
67.0563		0.000138					
69.0957		9.89E-05					

HYDRAULIC CONDUCTIVITY DATA HOMOGENEOUS PROFILE						
FITTED CURVE			LAYER 1			
THETA	K		THETA	K		
0.0016	4.58E-08		0.206514	0.099626		
0.0033	3.86E-07		0.121003	0.044497		
0.0065	3.24E-06		0.096177	0.028913		
0.013	2.73E-05		0.082385	0.017226		
0.0195	9.52E-05		0.068593	0.010142		
0.026	0.000231		0.060318	0.00702		
0.0326	0.00046		0.052043	0.004958		
0.0391	0.000807		0.049284	0.003831		
0.0456	0.001301		0.043767	0.003041		
0.0521	0.001968		0.03825	0.002123		
0.0586	0.002836		0.032734	0.001215		
0.0651	0.003936		0.029975	0.000827		
0.0716	0.005297		0.027217	0.000601		
0.0781	0.006951		0.0217	0.000148		
0.0846	0.008929		0.010666	9.48E-05		
0.0912	0.01127		0.007908	3.45E-05		
0.0977	0.01399		0.005149	1.49E-05		
0.1042	0.01715		0.005149	9.82E-06		
0.1107	0.02077					
0.1172	0.0249			LAYER 2		
0.1237	0.02956		THETA	K		
0.1302	0.03482		0.242374	0.251597		
0.1367	0.04069		0.134795	0.083651		
0.1432	0.04725		0.107211	0.045977		
0.1498	0.05452		0.096177	0.027294		
0.1563	0.06256		0.082385	0.015166		
0.1628	0.07142		0.07411	0.010206		
0.1693	0.08116		0.057559	0.007091		
0.1758	0.09184		0.057559	0.005425		
0.1823	0.1035		0.052043	0.004278		
0.1888	0.1163		0.043767	0.002962		
0.1953	0.1301		0.03825	0.001686		
0.2018	0.1452		0.032734	0.001148		
0.2084	0.1616		0.029975	0.000836		
0.2149	0.1794		0.024458	0.000263		
0.2214	0.1987		0.018941	0.000181		
0.2279	0.2195		0.013425	7.17E-05		
0.2344	0.2421		0.007908	3.86E-05		
0.2409	0.2666		0.007908	2.56E-05		
0.2474	0.2931					
0.2539	0.3218					
0.2604	0.353					
0.267	0.3869					
0.2735	0.4239					
0.28	0.4644					

0.2865		0.5091					
0.293		0.559					
0.2995		0.6155					
0.306		0.6824					
0.3093		0.7227					
0.3093		0.78					
	LAYER 3				LAYER 5		
THETA		K		THETA		K	
0.228581		0.346965		0.308576		0.716505	
0.159621		0.116261		0.217548		0.215001	
0.132037		0.061893		0.134795		0.105408	
0.109969		0.037295		0.112728		0.063108	
0.096177		0.020519		0.098936		0.033465	
0.09066		0.013735		0.09066		0.021965	
0.076868		0.009517		0.082385		0.014991	
0.071351		0.007272		0.068593		0.0114	
0.065835		0.005731		0.065835		0.008936	
0.063076		0.00397		0.065835		0.006148	
0.049284		0.002222		0.057559		0.003446	
0.043767		0.001549		0.054801		0.002383	
0.03825		0.001133		0.049284		0.001743	
0.032734		0.000422		0.041009		0.0008	
0.029975		0.000306		0.035492		0.000616	
0.0217		0.00013		0.029975		0.000262	
0.016183		8.14E-05		0.032734		0.000206	
0.016183		5.62E-05		0.032734		0.000217	
	LAYER 4				LAYER 6		
THETA		K		THETA		K	
0.289267		0.542723		0.316851		0.856688	
0.198239		0.165362		0.256166		0.259525	
0.118245		0.083012		0.192722		0.12659	
0.093419		0.049659		0.170655		0.076132	
0.082385		0.026632		0.145829		0.040156	
0.079627		0.01759		0.134795		0.026215	
0.063076		0.01209					
0.063076		0.009188					
0.057559		0.007215					
0.054801		0.004974					
0.046526		0.002784					
0.041009		0.001931					
0.041009		0.001411					
0.032734		0.00059					
0.027217		0.000442					
0.0217		0.000198					
0.0217		0.000138					
0.0217		9.89E-05					

APPENDIX D

EXPERIMENTAL DATA: HETEROGENEOUS SOIL PROFILE

SEVILLETA SAND - 6.5 CM STONE LAYER

CORRECTED TENSIO METER READINGS AT CUPS						
MINUTES	T1	T2	T3	T4	T5	
1	-20.66	-20.1501	-14.0319	-19.1304	-17.091	
15	-33.9161	-30.3471	-23.2092	-25.2486	-22.6994	
30	-37.485	-33.4062	-27.288	-28.3077	-25.7585	
45	-40.0343	-35.9555	-30.3471	-31.8767	-29.3274	
60	-42.0737	-39.0146	-33.4062	-33.4062	-30.3471	
75	-42.5835	-40.0343	-34.9358	-34.9358	-32.3865	
90	-44.6229	-41.054	-35.9555	-35.4456	-34.4259	
120	-47.1722	-43.0934	-38.5047	-38.5047	-35.4456	
155	-49.2116	-44.6229	-39.5244	-40.5441	-37.485	
330	-54.3101	-50.2313	-45.1328	-46.1525	-43.6032	
450	-56.3495	-52.2707	-47.1722	-48.1919	-45.6426	
560	-57.879	-54.8199	-48.1919	-49.7214	-47.682	
1220	-64.5071	-59.9184	-54.3101	-55.8396	-53.8002	
2030	-68.076	-64.5071	-58.3889	-60.4283	-58.3889	
2955	-71.1351	-67.0563	-59.9184	-62.9775	-61.9578	
HYDRAULIC HEAD AT TENSIO METER CUPS H						
MINUTES	T1	T2	T3	T4	T5	
1	-31.66	-41.1501	-45.7819	-60.8804	-68.841	
15	-44.9161	-51.3471	-54.9592	-66.9986	-74.4494	
30	-48.485	-54.4062	-59.038	-70.0577	-77.5085	
45	-51.0343	-56.9555	-62.0971	-73.6267	-81.0774	
60	-53.0737	-60.0146	-65.1562	-75.1562	-82.0971	
75	-53.5835	-61.0343	-66.6858	-76.6858	-84.1365	
90	-55.6229	-62.054	-67.7055	-77.1956	-86.1759	
120	-58.1722	-64.0934	-70.2547	-80.2547	-87.1956	
155	-60.2116	-65.6229	-71.2744	-82.2941	-89.235	
330	-65.3101	-71.2313	-76.8828	-87.9025	-95.3532	
450	-67.3495	-73.2707	-78.9222	-89.9419	-97.3926	
560	-68.879	-75.8199	-79.9419	-91.4714	-99.432	
1220	-75.5071	-80.9184	-86.0601	-97.5896	-105.55	
2030	-79.076	-85.5071	-90.1389	-102.178	-110.139	
2955	-82.1351	-88.0563	-91.6684	-104.728	-113.708	

SLOPE OF THE WETTNESS CURVE dTH/dt						
MINUTES	dTH/dt 1	dTH/dt 2	dTH/dt 3	dTH/dt 4	dTH/dt 5	
1	-0.01673	-0.00371	-0.00073	-0.00219	-0.00237	
15	-0.00213	-0.00272	-0.00134	-0.00104	-0.00111	
30	-0.00111	-0.00203	-0.00135	-0.00071	-0.00075	
45	-0.00074	-0.00157	-0.00126	-0.00054	-0.00057	
60	-0.00054	-0.00125	-0.00113	-0.00044	-0.00046	
75	-0.00043	-0.00101	-0.001	-0.00037	-0.00038	
90	-0.00035	-0.00084	-0.00088	-0.00032	-0.00033	
120	-0.00025	-0.0006	-0.00069	-0.00024	-0.00025	
155	-0.00019	-0.00043	-0.00052	-0.00019	-0.0002	
330	-7.4E-05	-0.00014	-0.00017	-8.6E-05	-8.8E-05	
450	-5E-05	-8.4E-05	-9.8E-05	-6E-05	-6.1E-05	
560	-3.8E-05	-5.8E-05	-6.5E-05	-4.6E-05	-4.6E-05	
1220	-1.3E-05	-1.7E-05	-1.3E-05	-1.7E-05	-1.7E-05	
2030	-6.6E-06	-8.8E-06	-4.3E-06	-8.2E-06	-8.1E-06	
2955	-3.9E-06	-5.6E-06	-1.9E-06	-4.8E-06	-4.7E-06	
WATER CONTENT dTH/dt TIMES THE THICKNESS OF THE LAYER						
MINUTES	T1	T2	T3	T4	T5	
1	0.334615	0.037113	0.007267	0.02189	0.023724	
15	0.042667	0.027181	0.013365	0.010367	0.011093	
30	0.022192	0.020302	0.013539	0.007065	0.007499	
45	0.014726	0.015688	0.012563	0.005409	0.005708	
60	0.010873	0.012457	0.011288	0.004383	0.004604	
75	0.008534	0.010115	0.010008	0.003677	0.003848	
90	0.00697	0.008366	0.008828	0.003159	0.003295	
120	0.005025	0.005986	0.006868	0.002448	0.002541	
155	0.003726	0.004294	0.005194	0.001921	0.001984	
330	0.00148	0.001385	0.001711	0.000862	0.000879	
450	0.000998	0.000836	0.000981	0.0006	0.000608	
560	0.000752	0.000584	0.000646	0.000459	0.000464	
1220	0.000267	0.000173	0.00013	0.000166	0.000166	
2030	0.000133	8.76E-05	4.28E-05	8.17E-05	8.12E-05	
2955	7.85E-05	5.61E-05	1.86E-05	4.76E-05	4.71E-05	

SLOPE OF THE HEAD WITH DEPTH CURVE dH/dz						
MINUTES	G1	G2	G3	G4	G5	
1	0.949015	0.46318	1.50985	0.79606	0.724642	
15	0.643105	0.36121	1.20394	0.745075	0.783677	
30	0.59212	0.46318	1.10197	0.745075	0.815878	
45	0.59212	0.514165	1.152955	0.745075	0.853446	
60	0.69409	0.514165	1	0.69409	0.86418	
75	0.745075	0.56515	1	0.745075	0.885647	
90	0.643105	0.56515	0.949015	0.89803	0.907115	
120	0.59212	0.616135	1	0.69409	0.917848	
155	0.541135	0.56515	1.10197	0.69409	0.939316	
330	0.59212	0.56515	1.10197	0.745075	1.003718	
450	0.59212	0.56515	1.10197	0.745075	1.025185	
560	0.69409	0.412195	1.152955	0.79606	1.046653	
1220	0.541135	0.514165	1.152955	0.79606	1.111055	
2030	0.643105	0.46318	1.20394	0.79606	1.159356	
2955	0.59212	0.36121	1.30591	0.89803	1.196924	
FLUX THROUGH EACH LAYER						
MINUTES	Q1	Q2	Q3	Q4	Q5	
1	0.334615	0.371728	0.378995	0.400885	0.424609	
15	0.042667	0.069848	0.083213	0.09358	0.104673	
30	0.022192	0.042493	0.056033	0.063098	0.070598	
45	0.014726	0.030414	0.042977	0.048386	0.054094	
60	0.010873	0.02333	0.034619	0.039002	0.043606	
75	0.008534	0.018648	0.028656	0.032333	0.036181	
90	0.00697	0.015337	0.024164	0.027324	0.030619	
120	0.005025	0.011011	0.017879	0.020328	0.022869	
155	0.003726	0.00802	0.013214	0.015135	0.01712	
330	0.00148	0.002865	0.004576	0.005439	0.006317	
450	0.000998	0.001834	0.002815	0.003415	0.004022	
560	0.000752	0.001336	0.001982	0.002441	0.002904	
1220	0.000267	0.00044	0.000569	0.000736	0.000902	
2030	0.000133	0.00022	0.000263	0.000345	0.000426	
2955	7.85E-05	0.000135	0.000153	0.000201	0.000248	

SOIL WATER RETENTION DATA 6.5 CM STONE LAYER						
FITTED CURVE				LAYER 1		
THETA		TENSION		THETA		TENSION
0.0016		136.8		0.218927		20.65995
0.0033		112.2		0.172034		33.91605
0.0065		91.96		0.14445		37.485
0.013		75.28		0.127899		40.03425
0.0195		66.87		0.111349		42.07365
0.026		61.41		0.100315		42.5835
0.0326		57.44		0.096177		44.6229
0.0391		54.34		0.085144		47.17215
0.0456		51.81		0.078248		49.21155
0.0521		49.68		0.058939		54.31005
0.0586		47.84		0.054801		56.34945
0.0651		46.22		0.049284		57.879
0.0716		44.78		0.036871		64.50705
0.0781		43.48		0.031354		68.076
0.0846		42.28		0.029975		71.1351
0.0912		41.19				
0.0977		40.17			LAYER 2	
0.1042		39.21		THETA		TENSION
0.1107		38.31		0.359606		ND
0.1172		37.46		0.343056		20.1501
0.1237		36.65		0.308576		30.3471
0.1302		35.88		0.272716		33.4062
0.1367		35.13		0.238236		35.95545
0.1432		34.41		0.216169		39.01455
0.1498		33.72		0.203756		40.03425
0.1563		33.04		0.184447		41.05395
0.1628		32.39		0.167896		43.09335
0.1693		31.74		0.130657		44.6229
0.1758		31.11		0.121003		50.23125
0.1823		30.48		0.112728		52.27065
0.1888		29.86		0.09066		54.8199
0.1953		29.25		0.079627		59.9184
0.2018		28.63		ND		64.50705
0.2084		28.02				
0.2149		27.4				
0.2214		26.78				
0.2279		26.14				
0.2344		25.5				
0.2409		24.83				
0.2474		24.15				
0.2539		23.43				
0.2604		22.68				
0.267		21.88				
0.2735		21.01				
0.28		20.05				

0.2865		18.97					
0.293		17.7					
0.2995		16.08					
0.306		13.71					
0.3093		11.72					
	LAYER 3				LAYER 5		
THETA		TENSION		THETA		TENSION	
0.143204		14.0319		0.172034		17.091	
0.13948		23.2092		0.159621		22.69935	
0.137618		27.288		0.149966		25.75845	
0.128308		30.3471		0.129278		29.3274	
0.112171		33.4062		0.1225		30.3471	
0.105344		34.93575		0.115486		32.3865	
0.096655		35.95545		0.111349		34.4259	
0.085484		38.5047		0.103073		35.4456	
0.078656		39.5244		0.096177		37.485	
0.059417		45.13275		0.072731		43.6032	
0.05321		47.17215		0.065835		45.6426	
0.050107		48.19185		0.058939		47.682	
0.037073		54.31005		0.041009		53.8002	
0.032729		58.38885		0.032734		58.38885	
0.029005		59.9184		0.025838		61.9578	
	LAYER 4						
THETA		TENSION					
0.167896		19.1304					
0.155483		25.2486					
0.147208		28.3077					
0.132037		31.87665					
0.123		33.4062					
0.114107		34.93575					
0.111349		35.4456					
0.098936		38.5047					
0.093419		40.5441					
0.071351		46.15245					
0.061697		48.19185					
0.060318		49.7214					
0.045146		55.8396					
0.036871		60.42825					
0.025838		62.9775					

HYDRAULIC CONDUCTIVITY TENSION DATA 6.5 CM STONE LAYER					
FITTED CURVE			LAYER 1		
TENSION	K		TENSION	K	
123.6	9.47E-08		20.65995	0.352592	
103.1	7.97E-07		33.91605	0.066345	
86.02	6.7E-06		37.485	0.037478	
71.64	5.65E-05		40.03425	0.024871	
64.29	0.000197		42.07365	0.015665	
59.47	0.000477		42.5835	0.011453	
55.94	0.00095		44.6229	0.010838	
53.17	0.001668		47.17215	0.008486	
50.9	0.002687		49.21155	0.006885	
48.97	0.004064		54.31005	0.0025	
47.31	0.005857		56.34945	0.001685	
45.84	0.008127		57.879	0.001083	
44.52	0.01093		64.50705	0.000493	
43.33	0.01435		68.076	0.000206	
42.24	0.01842		71.1351	0.000133	
41.23	0.02324				
40.28	0.02886			LAYER 2	
39.4	0.03536		TENSION	K	
38.57	0.04281		20.1501	0.802556	
37.78	0.0513		30.3471	0.193372	
37.03	0.06089		33.4062	0.091742	
36.3	0.07168		35.95545	0.059153	
35.61	0.08375		39.01455	0.045375	
34.94	0.09719		40.03425	0.032997	
34.29	0.1121		41.05395	0.027137	
33.65	0.1286		43.09335	0.017871	
33.03	0.1467		44.6229	0.014191	
32.42	0.1667		50.23125	0.00507	
31.82	0.1885		52.27065	0.003246	
31.23	0.2123		54.8199	0.003241	
30.64	0.2383		59.9184	0.000855	
30.06	0.2666		64.50705	0.000475	
29.47	0.2973				
28.89	0.3307				
28.3	0.3668				
27.7	0.4059				
27.09	0.4482				
26.47	0.4939				
25.83	0.5433				
25.17	0.5967				
24.47	0.6545				
23.74	0.7171				
22.96	0.7849				
22.12	0.8588				
21.18	0.9394				

20.12		1.028					
18.85		1.126					
17.25		1.237					
14.87		1.366					
12.85		1.444					
	LAYER 3				LAYER 5		
TENSION		K			TENSION		K
14.0319		0.251015			17.091		0.585957
23.2092		0.069117			22.69935		0.133567
27.288		0.050848			25.75845		0.086529
30.3471		0.037276			29.3274		0.063383
33.4062		0.034619			30.3471		0.050459
34.93575		0.028656			32.3865		0.040853
35.95545		0.025463			34.4259		0.033754
38.5047		0.017879			35.4456		0.024916
39.5244		0.011991			37.485		0.018226
45.13275		0.004153			43.6032		0.006294
47.17215		0.002555			45.6426		0.003924
48.19185		0.001719			47.682		0.002775
54.31005		0.000494			53.8002		0.000811
58.38885		0.000218			58.38885		0.000367
59.9184		0.000117			61.9578		0.000207
	LAYER 4						
TENSION		K					
19.1304		0.503586					
25.2486		0.125599					
28.3077		0.084687					
31.87665		0.064941					
33.4062		0.056191					
34.93575		0.043396					
35.4456		0.030426					
38.5047		0.029287					
40.5441		0.021806					
46.15245		0.0073					
48.19185		0.004583					
49.7214		0.003066					
55.8396		0.000924					
60.42825		0.000433					
62.9775		0.000224					

HYDRAULIC CONDUCTIVITY DATA 6.5 CM STONE LAYER				
FITTED CURVE			LAYER 1	
THETA		K	THETA	K
0.0016		4.58E-08	0.218927	0.352592
0.0033		3.86E-07	0.172034	0.066345
0.0065		3.24E-06	0.14445	0.037478
0.013		2.73E-05	0.127899	0.024871
0.0195		9.52E-05	0.111349	0.015665
0.026		0.000231	0.100315	0.011453
0.0326		0.00046	0.096177	0.010838
0.0391		0.000807	0.085144	0.008486
0.0456		0.001301	0.078248	0.006885
0.0521		0.001968	0.058939	0.0025
0.0586		0.002836	0.054801	0.001685
0.0651		0.003936	0.049284	0.001083
0.0716		0.005297	0.036871	0.000493
0.0781		0.006951	0.031354	0.000206
0.0846		0.008929	0.029975	0.000133
0.0912		0.01127		
0.0977		0.01399		
0.1042		0.01715		
			THETA	K
0.1107		0.02077	0.359606	0.802556
0.1172		0.0249	0.343056	0.193372
0.1237		0.02956	0.308576	0.091742
0.1302		0.03482	0.272716	0.059153
0.1367		0.04069	0.238236	0.045375
0.1432		0.04725	0.216169	0.032997
0.1498		0.05452	0.203756	0.027137
0.1563		0.06256	0.184447	0.017871
0.1628		0.07142	0.167896	0.014191
0.1693		0.08116	0.130657	0.00507
0.1758		0.09184	0.121003	0.003246
0.1823		0.1035	0.112728	0.003241
0.1888		0.1163	0.09066	0.000855
0.1953		0.1301	0.079627	0.000475
0.2018		0.1452	0.07411	0.000372
0.2084		0.1616		
0.2149		0.1794		
0.2214		0.1987		
0.2279		0.2195		
0.2344		0.2421		
0.2409		0.2666		
0.2474		0.2931		
0.2539		0.3218		
0.2604		0.353		
0.267		0.3869		
0.2735		0.4239		
0.28		0.4644		

0.2865		0.5091					
0.293		0.559					
0.2995		0.6155					
0.306		0.6824					
0.3093		0.7227					
0.3093		0.78					
	LAYER 3				LAYER 5		
THETA		K			THETA		K
0.143204		0.251015			0.172034		0.585957
0.13948		0.069117			0.159621		0.133567
0.137618		0.050848			0.149966		0.086529
0.128308		0.037276			0.129278		0.063383
0.112171		0.034619			ND		0.050459
0.105344		0.028656			0.115486		0.040853
0.096655		0.025463			0.111349		0.033754
0.085484		0.017879			0.103073		0.024916
0.078656		0.011991			0.096177		0.018226
0.059417		0.004153			0.072731		0.006294
0.05321		0.002555			0.065835		0.003924
0.050107		0.001719			0.058939		0.002775
0.037073		0.000494			0.041009		0.000811
0.032729		0.000218			0.032734		0.000367
0.029005		0.000117			0.025838		0.000207
	LAYER 4						
THETA		K					
0.167896		0.503586					
0.155483		0.125599					
0.147208		0.084687					
0.132037		0.064941					
ND		0.056191					
0.114107		0.043396					
0.111349		0.030426					
0.098936		0.029287					
0.093419		0.021806					
0.071351		0.0073					
0.061697		0.004583					
0.060318		0.003066					
0.045146		0.000924					
0.036871		0.000433					
0.025838		0.000224					

APPENDIX E

EXPERIMENTAL DATA: HETEROGENEOUS SOIL PROFILE

SEVILLETA SAND - 10 CM STONE LAYER

EXPERIMENT #14 HETEROGENEOUS SOIL PROFILE: 10 CM STONE LAYER						
TENSIO METER READINGS mbar						
MINUTES	T1	T2	T3	T4	T5	
0	-36	-31.5	-28	-33.5	-32	
15	-46.5	-40	-37	-38.5	-34.5	
30	-49.5	-46.5	-42	-41	-38.5	
45	-54.5	-49.5	-45	-45.5	-41	
60	-55.5	-51.5	-47	-45	-43.5	
80	-58	-53	-51	-50.5	-54.5	
105	-60.5	-55.5	-53	-52.5	-48.5	
135	-61.5	-58	-55	-54.5	-50	
300	-67.5	-63	-61	-57.5	-55.5	
360	-69	-64	-62	-62	-57	
570	-72.5	-68	-65	-64.5	-61.5	
825	-75	-71	-68.5	-68.5	-64	
1575	-81	-77	-75	-74	-69.5	
3000	-83.5	-78.5	-77	-76	-73	
4912	-89.5	-85	-83	-82.5	-79.5	
TENSIO METER READINGS cm H2O						
MINUTES	T1	T2	T3	T4	T5	
0	-36.7092	-32.1206	-28.5516	-34.16	-32.6304	
15	-47.4161	-40.788	-37.7289	-39.2585	-35.1797	
30	-50.4752	-47.4161	-42.8274	-41.8077	-39.2585	
45	-55.5737	-50.4752	-45.8865	-46.3964	-41.8077	
60	-56.5934	-52.5146	-47.9259	-45.8865	-44.357	
80	-59.1426	-54.0441	-52.0047	-51.4949	-55.5737	
105	-61.6919	-56.5934	-54.0441	-53.5343	-49.4555	
135	-62.7116	-59.1426	-56.0835	-55.5737	-50.985	
300	-68.8298	-64.2411	-62.2017	-58.6328	-56.5934	
360	-70.3593	-65.2608	-63.2214	-63.2214	-58.1229	
570	-73.9283	-69.3396	-66.2805	-65.7707	-62.7116	
825	-76.4775	-72.3987	-69.8495	-69.8495	-65.2608	
1575	-82.5957	-78.5169	-76.4775	-75.4578	-70.8692	
3000	-85.145	-80.0465	-78.5169	-77.4972	-74.4381	
4912	-91.2632	-86.6745	-84.6351	-84.1253	-81.0662	

CORRECTED TENSIO METER READINGS AT CUPS						
MINUTES	T1	T2	T3	T4	T5	
0	-23.2092	-18.6206	-15.0516	-20.66	-19.1304	
15	-33.9161	-27.288	-24.2289	-25.7585	-21.6797	
30	-36.9752	-33.9161	-29.3274	-28.3077	-25.7585	
45	-42.0737	-36.9752	-32.3865	-32.8964	-28.3077	
60	-43.0934	-39.0146	-34.4259	-32.3865	-30.857	
80	-45.6426	-40.5441	-38.5047	-37.9949	-42.0737	
105	-48.1919	-43.0934	-40.5441	-40.0343	-35.9555	
135	-49.2116	-45.6426	-42.5835	-42.0737	-37.485	
300	-55.3298	-50.7411	-48.7017	-45.1328	-43.0934	
360	-56.8593	-51.7608	-49.7214	-49.7214	-44.6229	
570	-60.4283	-55.8396	-52.7805	-52.2707	-49.2116	
825	-62.9775	-58.8987	-56.3495	-56.3495	-51.7608	
1575	-69.0957	-65.0169	-62.9775	-61.9578	-57.3692	
3000	-71.645	-66.5465	-65.0169	-63.9972	-60.9381	
4912	-77.7632	-73.1745	-71.1351	-70.6253	-67.5662	
HYDRAULIC HEAD AT TENSIO METER CUPS cm						
MINUTES	T1	T2	T3	T4	T5	
0	-33.7092	-39.8706	-46.5516	-62.16	-70.6304	
15	-44.4161	-48.538	-55.7289	-67.2585	-73.1797	
30	-47.4752	-55.1661	-60.8274	-69.8077	-77.2585	
45	-52.5737	-58.2252	-63.8865	-74.3964	-79.8077	
60	-53.5934	-60.2646	-65.9259	-73.8865	-82.357	
80	-56.1426	-61.7941	-70.0047	-79.4949	-93.5737	
105	-58.6919	-64.3434	-72.0441	-81.5343	-87.4555	
135	-59.7116	-66.8926	-74.0835	-83.5737	-88.985	
300	-65.8298	-71.9911	-80.2017	-86.6328	-94.5934	
360	-67.3593	-73.0108	-81.2214	-91.2214	-96.1229	
570	-70.9283	-77.0896	-84.2805	-93.7707	-100.712	
825	-73.4775	-80.1487	-87.8495	-97.8495	-103.261	
1575	-79.5957	-86.2669	-94.4775	-103.458	-108.869	
3000	-82.145	-87.7965	-96.5169	-105.497	-112.438	
4912	-88.2632	-94.4245	-102.635	-112.125	-119.066	

SLOPE OF THE HEAD WITH DEPTH CURVE dH/dt						
MINUTES	G1	G2	G3	G4	G5	
0	0.616135	0.668105	1.560835	0.847045	0.743478	
15	0.412195	0.71909	1.152955	0.59212	0.770312	
30	0.76909	0.566135	0.89803	0.745075	0.813247	
45	0.56515	0.566135	1.050985	0.541135	0.840081	
60	0.66712	0.566135	0.79606	0.847045	0.866915	
80	0.56515	0.82106	0.949015	1.40788	0.984986	
105	0.56515	0.770075	0.949015	0.59212	0.920584	
135	0.718105	0.71909	0.949015	0.541135	0.936684	
300	0.616135	0.82106	0.643105	0.79606	0.995719	
360	0.56515	0.82106	1	0.49015	1.01182	
570	0.616135	0.71909	0.949015	0.69409	1.060122	
825	0.66712	0.770075	1	0.541135	1.086956	
1575	0.66712	0.82106	0.89803	0.541135	1.145991	
3000	0.56515	0.872045	0.89803	0.69409	1.183559	
4912	0.616135	0.82106	0.949015	0.69409	1.253328	
FLUX THROUGH EACH LAYER						
MINUTES	Q1	Q2	Q3	Q4	Q5	
0	-3.2E-06	0.069356	0.076302	-89.0871	-89.0651	
15	0.035241	0.06443	0.070641	0.07626	0.086778	
30	0.018882	0.037415	0.042927	0.047417	0.054636	
45	0.012627	0.026125	0.031019	0.034829	0.040384	
60	0.009354	0.019878	0.024225	0.027559	0.032078	
80	0.006852	0.014895	0.018611	0.021478	0.02509	
105	0.005057	0.011182	0.014241	0.016683	0.019556	
135	0.003789	0.008478	0.010908	0.012977	0.015268	
300	0.001443	0.003276	0.004032	0.005103	0.006119	
360	0.001146	0.0026	0.003127	0.004021	0.004849	
570	0.000632	0.001423	0.001645	0.002183	0.002663	
825	0.000386	0.00086	0.001009	0.001348	0.001649	
1575	0.000157	0.000344	0.000443	0.000579	0.000706	
3000	6.2E-05	0.000133	0.000184	0.000233	0.000284	
4912	2.96E-05	6.31E-05	8.36E-05	0.000105	0.00013	

SOIL WATER RETENTION DATA 10 CM STONE LAYER						
FITTED CURVE				LAYER 1		
THETA	TENSION		THETA	TENSION		
0.0016	136.8		0.170655	23.2092		
0.0033	112.2		0.130657	33.91605		
0.0065	91.96		0.112728	36.97515		
0.013	75.28		0.097556	42.07365		
0.0195	66.87		0.09066	43.09335		
0.026	61.41		0.085144	45.6426		
0.0326	57.44		0.076868	48.19185		
0.0391	54.34		0.068593	49.21155		
0.0456	51.81		0.052043	55.32975		
0.0521	49.68		0.045146	56.8593		
0.0586	47.84		0.03825	60.42825		
0.0651	46.22		0.031354	62.9775		
0.0716	44.78		0.023079	69.0957		
0.0781	43.48		0.013425	71.64495		
0.0846	42.28		0.010666	77.76315		
0.0912	41.19					
0.0977	40.17			LAYER 2		
0.1042	39.21		THETA	TENSION		
0.1107	38.31		0.337539	18.62055		
0.1172	37.46		0.319609	27.288		
0.1237	36.65		0.285129	33.91605		
0.1302	35.88		0.245132	36.97515		
0.1367	35.13		0.218927	39.01455		
0.1432	34.41		0.189964	40.5441		
0.1498	33.72		0.170655	43.09335		
0.1563	33.04		0.155483	45.6426		
0.1628	32.39		0.118245	50.7411		
0.1693	31.74		0.107211	51.7608		
0.1758	31.11		0.093419	55.8396		
0.1823	30.48		0.081006	58.8987		
0.1888	29.86		0.067214	65.0169		
0.1953	29.25		0.032734	66.54645		
0.2018	28.63		0.029975	73.1745		
0.2084	28.02					
0.2149	27.4					
0.2214	26.78					
0.2279	26.14					
0.2344	25.5					
0.2409	24.83					
0.2474	24.15					
0.2539	23.43					
0.2604	22.68					
0.267	21.88					
0.2735	21.01					
0.28	20.05					

0.2865		18.97				
0.293		17.7				
0.2995		16.08				
0.306		13.71				
0.3093		11.72				
	LAYER 3				LAYER 5	
THETA		TENSION		THETA		TENSION
0.12086		15.0516		0.173413		19.1304
0.117136		24.2289		0.163759		21.67965
0.113413		29.3274		0.151346		25.75845
0.104103		32.3865		0.141691		28.3077
0.094173		34.4259		0.134795		30.85695
0.084863		38.5047		0.12652		42.07365
0.073691		40.5441		0.116865		35.95545
0.065002		42.5835		0.10859		37.485
0.046383		48.7017		0.081006		43.09335
0.042038		49.7214		0.072731		44.6229
0.035211		52.7805		0.060318		49.21155
0.029626		56.34945		0.050663		51.7608
0.022178		62.9775		0.041009		57.36915
0.007903		65.0169		0.024458		60.9381
0.006662		71.1351		0.023079		67.56615
	LAYER 4					
THETA		TENSION				
0.152725		20.65995				
0.14307		25.75845				
0.134795		28.3077				
0.130657		32.89635				
0.122382		32.3865				
0.116865		37.99485				
0.105832		40.03425				
0.103073		42.07365				
0.075489		45.13275				
0.068593		49.7214				
0.057559		52.27065				
0.046526		56.34945				
0.031354		61.9578				
0.020321		63.9972				
0.017562		70.62525				

HYDRAULIC CONDUCTIVITY TENSION DATA 10 CM STONE LAYER						
FITTED CURVE			LAYER 1			
TENSION		K		TENSION		K
123.6		9.47E-08		23.2092		ND
103.1		7.97E-07		33.91605		0.085496
86.02		6.7E-06		36.97515		0.024551
71.64		5.65E-05		42.07365		0.022343
64.29		0.000197		43.09335		0.014021
59.47		0.000477		45.6426		0.012125
55.94		0.00095		48.19185		0.008948
53.17		0.001668		49.21155		0.005276
50.9		0.002687		55.32975		0.002342
48.97		0.004064		56.8593		0.002029
47.31		0.005857		60.42825		0.001026
45.84		0.008127		62.9775		0.000578
44.52		0.01093		69.0957		0.000236
43.33		0.01435		71.64495		0.00011
42.24		0.01842		77.76315		4.81E-05
41.23		0.02324				
40.28		0.02886			LAYER 2	
39.4		0.03536		TENSION		K
38.57		0.04281		18.62055		0.10381
37.78		0.0513		27.288		0.089599
37.03		0.06089		33.91605		0.066088
36.3		0.07168		36.97515		0.046146
35.61		0.08375		39.01455		0.035112
34.94		0.09719		40.5441		0.018141
34.29		0.1121		43.09335		0.01452
33.65		0.1286		45.6426		0.011789
33.03		0.1467		50.7411		0.00399
32.42		0.1667		51.7608		0.003167
31.82		0.1885		55.8396		0.001979
31.23		0.2123		58.8987		0.001117
30.64		0.2383		65.0169		0.000419
30.06		0.2666		66.54645		0.000153
29.47		0.2973		73.1745		7.69E-05
28.89		0.3307				
28.3		0.3668				
27.7		0.4059				
27.09		0.4482				
26.47		0.4939				
25.83		0.5433				
25.17		0.5967				
24.47		0.6545				
23.74		0.7171				
22.96		0.7849				
22.12		0.8588				
21.18		0.9394				

20.12		1.028					
18.85		1.126					
17.25		1.237					
14.87		1.366					
12.85		1.444					
	LAYER 3				LAYER 5		
TENSION		K			TENSION		K
15.0516		0.048886			19.1304		ND
24.2289		0.061269			21.67965		0.112654
29.3274		0.047801			25.75845		0.067182
32.3865		0.029514			28.3077		0.048072
34.4259		0.030431			30.85695		0.037003
38.5047		0.019611			33		0.025472
40.5441		0.015006			35.95545		0.021244
42.5835		0.011494			37.485		0.0163
48.7017		0.006269			43.09335		0.006146
49.7214		0.003127			44.6229		0.004792
52.7805		0.001733			49.21155		0.002512
56.34945		0.001009			51.7608		0.001517
62.9775		0.000494			57.36915		0.000616
65.0169		0.000205			60.9381		0.00024
71.1351		8.81E-05			67.56615		0.000103
	LAYER 4						
TENSION		K					
20.65995		0.128791					
25.75845		0.06364					
28.3077		0.064363					
32.89635		0.032535					
32.3865		0.015256					
37.99485		0.028175					
40.03425		0.023981					
42.07365		0.00641					
45.13275		0.008203					
49.7214		0.003145					
52.27065		0.00249					
56.34945		0.001071					
61.9578		0.000336					
63.9972		0.000151					
70.62525		ND					

HYDRAULIC CONDUCTIVITY DATA 10 CM STONE LAYER					
FITTED CURVE			LAYER 1		
THETA	K		THETA	K	
0.0016	4.58E-08		0.170655	ND	
0.0033	3.86E-07		0.130657	0.085496	
0.0065	3.24E-06		0.112728	0.024551	
0.013	2.73E-05		0.097556	0.022343	
0.0195	9.52E-05		0.09066	0.014021	
0.026	0.000231		0.085144	0.012125	
0.0326	0.00046		0.076868	0.008948	
0.0391	0.000807		0.068593	0.005276	
0.0456	0.001301		0.052043	0.002342	
0.0521	0.001968		0.045146	0.002029	
0.0586	0.002836		0.03825	0.001026	
0.0651	0.003936		0.031354	0.000578	
0.0716	0.005297		0.023079	0.000236	
0.0781	0.006951		0.013425	0.00011	
0.0846	0.008929		0.010666	4.81E-05	
0.0912	0.01127				
0.0977	0.01399			LAYER 2	
0.1042	0.01715		THETA	K	
0.1107	0.02077		0.337539	0.10381	
0.1172	0.0249		0.319609	0.089599	
0.1237	0.02956		0.285129	0.066088	
0.1302	0.03482		0.245132	0.046146	
0.1367	0.04069		0.218927	0.035112	
0.1432	0.04725		0.189964	0.018141	
0.1498	0.05452		0.170655	0.01452	
0.1563	0.06256		0.155483	0.011789	
0.1628	0.07142		0.118245	0.00399	
0.1693	0.08116		0.107211	0.003167	
0.1758	0.09184		0.093419	0.001979	
0.1823	0.1035		0.081006	0.001117	
0.1888	0.1163		0.067214	0.000419	
0.1953	0.1301		0.032734	0.000153	
0.2018	0.1452		0.029975	7.69E-05	
0.2084	0.1616				
0.2149	0.1794				
0.2214	0.1987				
0.2279	0.2195				
0.2344	0.2421				
0.2409	0.2666				
0.2474	0.2931				
0.2539	0.3218				
0.2604	0.353				
0.267	0.3869				
0.2735	0.4239				
0.28	0.4644				

0.2865		0.5091					
0.293		0.559					
0.2995		0.6155					
0.306		0.6824					
0.3093		0.7227					
0.3093		0.78					
	LAYER 3				LAYER 5		
THETA		K			THETA		K
0.12086		0.048886			0.173413		ND
0.117136		0.061269			0.163759		0.112654
0.113413		0.047801			0.151346		0.067182
0.104103		0.029514			0.141691		0.048072
0.094173		0.030431			0.134795		0.037003
0.084863		0.019611			0.12652		0.025472
0.073691		0.015006			0.116865		0.021244
0.065002		0.011494			0.10859		0.0163
0.046383		0.006269			0.081006		0.006146
0.042038		0.003127			0.072731		0.004792
0.035211		0.001733			0.060318		0.002512
0.029626		0.001009			0.050663		0.001517
0.022178		0.000494			0.041009		0.000616
0.007903		0.000205			0.024458		0.00024
0.006662		8.81E-05			0.023079		0.000103
	LAYER 4						
THETA		K					
0.152725		ND					
0.14307		0.128791					
0.134795		0.06364					
0.130657		0.064363					
0.122382		0.032535					
0.116865		0.015256					
0.105832		0.028175					
0.103073		0.023981					
0.075489		0.00641					
0.068593		0.008203					
0.057559		0.003145					
0.046526		0.00249					
0.031354		0.001071					
0.020321		0.000336					
0.017562		0.000151					

APPENDIX F

EXPERIMENTAL DATA: HETEROGENEOUS PROFILE

SEVILLETA SAND - 16 CM STONE LAYER

CORRECTED TENSIO METER READINGS AT CUPS cm							
RUNTIME	T1	T2	T3	T4	T5		
1	-11.8293	-7.9512	-5.8419	-12.2245	-16.9592		
15	-17.3874	-11.1982	-18.6818	-11.9845	-17.9272		
30	-35.1202	-28.0297	-21.0025	-20.2562	-21.7024		
45	-38.4927	-34.1166	-32.1063	-23.7245	-24.4516		
60	-41.1729	-35.2378	-35.6526	-26.9618	-27.5879		
75	-42.874	-39.3358	-37.9343	-29.0959	-30.095		
90	-44.5454	-41.6176	-39.3371	-30.322	-32.302		
105	-45.7915	-42.5743	-40.8405	-31.5483	-34.8576		
120	-46.8695	-41.612	-41.7803	-33.8075	-36.0482		
135	-47.8486	-41.993	-42.7196	-35.5343	-37.1033		
225	-51.6167	-47.3872	-46.4307	-41.1531	-39.5717		
330	-54.6035	-50.646	-49.1014	-44.6598	-47.1899		
705	-60.1419	-56.0335	-54.544	-53.7324	-55.2146		
4275	-74.1857	-68.1725	-67.7443	-66.8324	-69.3474		
7200	-75.7681	-70.1625	-69.8475	-68.1038	-67.8954		
HTDRAULIC HEAD AT TENSIO METER CUPS cm H2O							
PTD 1	PTD 2	PTD 3	PTD 4	PTD 5	PTD 6	PTD 7	PTD 8
-30.8293	-36.29	-37.6124	-76.9592	-38.0986	-52.5852	-65.3937	-57.5552
-36.3874	-40.2	-40.1963	-77.9272	-61.5399	-54.8236	-66.7545	-55.7144
-54.1202	-56.34	-57.7193	-81.7024	-54.0979	-66.907	-75.3404	-63.672
-57.4927	-61.96	-64.2731	-84.4516	-71.5283	-71.6842	-79.5109	-66.4381
-60.1729	-64.44	-64.0355	-87.5879	-75.2197	-75.0854	-82.8199	-69.6036
-61.874	-67.29	-69.3815	-90.095	-77.3122	-77.5563	-85.1206	-71.5711
-63.5454	-69.24	-71.9951	-92.302	-78.1314	-79.5428	-86.5793	-72.5646
-64.7915	-70.5	-72.6485	-94.8576	-79.8882	-80.7928	-88.1261	-73.4704
-65.8695	-71.09	-70.1339	-96.0482	-80.9344	-81.6261	-89.8491	-76.2658
-66.8486	-72.05	-69.9359	-97.1033	-82.4643	-81.9749	-90.9456	-78.6229
-70.6167	-75.76	-77.0144	-99.5717	-86.1655	-85.6959	-94.3232	-86.483
-73.6035	-79.07	-80.222	-107.19	-88.939	-88.2638	-98.2392	-89.5804
-79.1419	-84.4	-85.667	-115.215	-94.4366	-93.6514	-103.888	-102.077
-93.1857	-96.6	-97.745	-129.347	-107.465	-107.024	-117.105	-115.06
-94.7681	-98.6	-99.725	-127.895	-109.636	-109.059	-119.16	-115.547

WATER CONTENT dTH/dt TIMES THE THICKNESS OF THE LAYER						
RUNTIME	STAT 1	STAT 2	STAT 3	STAT 4	STAT 5	
1	0	-5.6E+16	0.004329	0.052422	-3.4E-06	
15	0.130409	0.065823	0.005781	0.009812	0.015815	
30	0.057447	0.010679	0.005449	0.005916	0.008881	
45	0.034339	0.011997	0.004908	0.004309	0.006134	
60	0.023576	0.009559	0.004359	0.003405	0.00466	
75	0.017518	0.007405	0.003858	0.002819	0.003741	
90	0.0137	0.005825	0.003415	0.002406	0.003114	
105	0.011106	0.004682	0.003031	0.002098	0.00266	
120	0.009245	0.003843	0.002699	0.001859	0.002315	
135	0.007855	0.003213	0.002412	0.001667	0.002046	
225	0.003828	0.001431	0.001333	0.001021	0.001179	
330	0.002203	0.000772	0.000768	0.000692	0.000769	
705	0.00071	0.000238	0.000209	0.000303	0.000318	
4275	3.78E-05	2.5E-05	5.42E-06	3.33E-05	3.28E-05	
7200	1.52E-05	1.48E-05	1.79E-06	1.66E-05	1.63E-05	
SLOPE OF THE HEAD WITH DEPTH CURVE dh/dz						
RUNTIME	G1	G2	G3	G4	G5	
1	0.61219	1.5634	1.28085	1.15655	0.810097	
15	0.381075	1.462545	1.19309	1.11727	0.820286	
30	0.290945	0.987735	0.84334	0.6362	0.860025	
45	0.562385	0.856765	0.78267	0.49407	0.888964	
60	0.406485	1.084765	0.77345	0.4768	0.921978	
75	0.646175	0.922055	0.75643	0.49744	0.948368	
90	0.707215	0.892525	0.70365	0.57227	0.9716	
105	0.678275	0.921855	0.73333	0.67315	0.998501	
120	0.474245	1.101415	0.8223	0.61991	1.011034	
135	0.414435	1.098195	0.89707	0.61577	1.02214	
225	0.57705	0.93087	0.86273	0.52485	1.048123	
330	0.60425	0.86178	0.99754	0.89507	1.128315	
705	0.58916	0.86179	1.02366	1.13266	1.212785	
4275	0.39868	0.98511	1.00809	1.22429	1.361552	
7200	0.43944	0.9896	1.01019	0.8735	1.346267	

SOIL WATER RETENTION DATA 16 CM STONE LAYER					
FITTED CURVE			LAYER 1		
THETA	TENSION		THETA	TENSION	
0.0016	136.8		0.320988	11.8293	
0.0033	112.2		0.31823	17.3874	
0.0065	91.96		0.257545	35.1202	
0.013	75.28		0.221685	38.4927	
0.0195	66.87		0.19686	41.1729	
0.026	61.41		0.181688	42.874	
0.0326	57.44		0.169275	44.5454	
0.0391	54.34		0.162379	45.7915	
0.0456	51.81		0.151346	46.8695	
0.0521	49.68		0.148587	47.8486	
0.0586	47.84		0.123761	51.6167	
0.0651	46.22		0.109969	54.6035	
0.0716	44.78		0.087902	60.1419	
0.0781	43.48		0.060318	74.1857	
0.0846	42.28		0.053422	75.7681	
0.0912	41.19				
0.0977	40.17				LAYER 2
0.1042	39.21		THETA	TENSION	
0.1107	38.31		0.14941	7.9512	
0.1172	37.46		0.147548	11.19815	
0.1237	36.65		0.135135	28.02965	
0.1302	35.88		0.125205	34.11655	
0.1367	35.13		0.11093	35.23775	
0.1432	34.41		0.102241	39.33575	
0.1498	33.72		0.094793	41.61755	
0.1563	33.04		0.090449	42.57425	
0.1628	32.39		0.085484	41.61195	
0.1693	31.74		0.083001	41.99295	
0.1758	31.11		0.069967	47.3872	
0.1823	30.48		0.061899	50.646	
0.1888	29.86		0.048866	56.0335	
0.1953	29.25		0.034591	68.1725	
0.2018	28.63		0.030246	70.1625	
0.2084	28.02				
0.2149	27.4				
0.2214	26.78				
0.2279	26.14				
0.2344	25.5				
0.2409	24.83				
0.2474	24.15				
0.2539	23.43				
0.2604	22.68				
0.267	21.88				
0.2735	21.01				
0.28	20.05				

0.2865		18.97					
0.293		17.7					
0.2995		16.08					
0.306		13.71					
0.3093		11.72					
	LAYER 3				LAYER 5		
THETA		TENSION			THETA		TENSION
0.135756		5.8419			0.141691		24.4516
0.133273		18.68175			0.132037		27.5879
0.128929		21.00245			0.12652		30.095
0.123343		32.10625			0.115486		32.302
0.115275		35.65255			0.111349		34.8576
0.106585		37.93425			0.105832		36.0482
0.100379		39.3371			0.103073		37.1033
0.096035		40.8405			0.087902		39.5717
0.089828		41.78025			0.076868		47.1899
0.087346		42.7196			0.063076		55.2146
0.07245		46.4307			0.03825		69.3474
0.060658		49.1014			0.029975		67.8954
0.050727		54.544					
0.03397		67.7443					
0.030867		69.84745					
	LAYER 4						
THETA		TENSION					
0.166517		12.22445					
0.14445		11.98445					
0.136174		20.2562					
0.129278		23.7245					
0.123761		26.96175					
0.119624		29.09585					
0.114107		30.32195					
0.111349		31.54825					
0.104453		33.80745					
0.105832		35.53425					
0.093419		41.1531					
0.083764		44.6598					
0.065835		53.7324					
0.035492		66.83235					
0.028596		68.1038					

HYDRAULIC CONDUCTIVITY TENSION DATA 16 CM STONE LAYER						
FITTED CURVE				LAYER 1		
TENSION	K			TENSION		K
123.6	9.47E-08			11.8293		ND
103.1	7.97E-07			17.3874		0.342214
86.02	6.7E-06			35.1202		0.197451
71.64	5.65E-05			38.4927		0.06106
64.29	0.000197			41.1729		0.058
59.47	0.000477			42.874		0.02711
55.94	0.00095			44.5454		0.019372
53.17	0.001668			45.7915		0.016373
50.9	0.002687			46.8695		0.019494
48.97	0.004064			47.8486		0.018954
47.31	0.005857			51.6167		0.006634
45.84	0.008127			54.6035		0.003646
44.52	0.01093			60.1419		0.001204
43.33	0.01435			74.1857		9.48E-05
42.24	0.01842			75.7681		3.45E-05
41.23	0.02324					
40.28	0.02886				LAYER 2	
				TENSION		K
39.4	0.03536			7.9512		ND
38.57	0.04281			11.19815		0.134171
37.78	0.0513			28.02965		0.068972
37.03	0.06089			34.11655		0.054082
36.3	0.07168			35.23775		0.030545
35.61	0.08375			39.33575		0.027029
34.94	0.09719			41.61755		0.021876
34.29	0.1121			42.57425		0.017126
33.65	0.1286			41.61195		0.011883
33.03	0.1467			41.99295		0.010079
32.42	0.1667			47.3872		0.00565
31.82	0.1885			50.646		0.003453
31.23	0.2123			56.0335		0.001099
30.64	0.2383			68.1725		6.38E-05
30.06	0.2666			70.1625		3.03E-05
29.47	0.2973					
28.89	0.3307					
28.3	0.3668					
27.7	0.4059					
27.09	0.4482					
26.47	0.4939					
25.83	0.5433					
25.17	0.5967					
24.47	0.6545					
23.74	0.7171					
22.96	0.7849					
22.12	0.8588					
21.18	0.9394					

20.12	1.028						
18.85	1.126						
17.25	1.237						
14.87	1.366						
12.85	1.444						
	LAYER 3				LAYER 5		
TENSION		K			TENSION		K
5.8419		ND			ND		0.277513
18.68175		0.169319			ND		0.102755
21.00245		0.087243			24.4516		0.069392
32.10625		0.065473			27.5879		0.049415
35.65255		0.048476			30.095		0.037264
37.93425		0.038047			32.302		0.029291
39.3371		0.032601			34.8576		0.023611
40.8405		0.025662			36.0482		0.019743
41.78025		0.019199			37.1033		0.016822
42.7196		0.015027			39.5717		0.008388
46.4307		0.007641			47.1899		0.004612
49.1014		0.003753			55.2146		0.001465
54.544		0.001129			69.3474		9.86E-05
67.7443		6.77E-05			67.8954		4.81E-05
69.84745		3.14E-05					
	LAYER 4						
TENSION		K					
12.22445		ND					
11.98445		0.189592					
20.2562		0.124947					
23.7245		0.112439					
26.96175		0.085779					
29.09585		0.063524					
30.32195		0.044289					
31.54825		0.031073					
33.80745		0.028465					
35.53425		0.0246					
41.1531		0.014504					
44.6598		0.004955					
53.7324		0.001288					
66.83235		8.29E-05					
68.1038		5.54E-05					

HYDRAULIC CONDUCTIVITY DATA 16 CM STONE LAYER						
FITTED CURVE				LAYER 1		
THETA		K		THETA		K
0.0016		4.58E-08		0.320988		ND
0.0033		3.86E-07		0.31823		0.342214
0.0065		3.24E-06		0.257545		0.197451
0.013		2.73E-05		0.221685		0.06106
0.0195		9.52E-05		0.19686		0.058
0.026		0.000231		0.181688		0.02711
0.0326		0.00046		0.169275		0.019372
0.0391		0.000807		0.162379		0.016373
0.0456		0.001301		0.151346		0.019494
0.0521		0.001968		0.148587		0.018954
0.0586		0.002836		0.123761		0.006634
0.0651		0.003936		0.109969		0.003646
0.0716		0.005297		0.087902		0.001204
0.0781		0.006951		0.060318		9.48E-05
0.0846		0.008929		0.053422		3.45E-05
0.0912		0.01127				
0.0977		0.01399			LAYER 2	
0.1042		0.01715		THETA		K
0.1107		0.02077		0.147548		0.134171
0.1172		0.0249		0.135135		0.068972
0.1237		0.02956		0.125205		0.054082
0.1302		0.03482		0.11093		0.030545
0.1367		0.04069		0.102241		0.027029
0.1432		0.04725		0.094793		0.021876
0.1498		0.05452		0.090449		0.017126
0.1563		0.06256		0.085484		0.011883
0.1628		0.07142		0.083001		0.010079
0.1693		0.08116		0.069967		0.00565
0.1758		0.09184		0.061899		0.003453
0.1823		0.1035		0.048866		0.001099
0.1888		0.1163		0.034591		6.38E-05
0.1953		0.1301		0.030246		3.03E-05
0.2018		0.1452				
0.2084		0.1616				
0.2149		0.1794				
0.2214		0.1987				
0.2279		0.2195				
0.2344		0.2421				
0.2409		0.2666				
0.2474		0.2931				
0.2539		0.3218				
0.2604		0.353				
0.267		0.3869				
0.2735		0.4239				
0.28		0.4644				

0.2865		0.5091					
0.293		0.559					
0.2995		0.6155					
0.306		0.6824					
0.3093		0.7227					
0.3093		0.78					
	LAYER 3				LAYER 5		
THETA		K			THETA		K
0.133273		0.169319			0.170655		ND
0.128929		0.087243			0.165138		0.277513
0.123343		0.065473			0.151346		0.102755
0.115275		0.048476			0.141691		0.069392
0.106585		0.038047			0.132037		0.049415
0.100379		0.032601			0.12652		0.037264
0.096035		0.025662			0.115486		0.029291
0.089828		0.019199			0.111349		0.023611
0.087346		0.015027			0.105832		0.019743
0.07245		0.007641			0.103073		0.016822
0.060658		0.003753			0.087902		0.008388
0.050727		0.001129			0.076868		0.004612
0.03397		6.77E-05			0.063076		0.001465
0.030867		3.14E-05			0.03825		9.86E-05
					0.029975		4.81E-05
	LAYER 4						
THETA		K					
0.166517		ND					
0.14445		0.189592					
0.136174		0.124947					
0.129278		0.112439					
0.123761		0.085779					
0.119624		0.063524					
0.114107		0.044289					
0.111349		0.031073					
0.104453		0.028465					
0.105832		0.0246					
0.093419		0.014504					
0.083764		0.004955					
0.065835		0.001288					
0.035492		8.29E-05					
0.028596		5.54E-05					